# Himmetdede Heap Leach Pad Prefeasibility Design

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SRK Consulting (U.S.), Inc. 216003 March 2012

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# **Executive Summary**

The Himmetdede Project is located in central Anatolia approximately 35 kilometers (km) northwest of Kayseri, along road D260, and adjacent to the village of Himmetdede. The Project is located at an elevation of approximately 1,200 meters (m) and the project rises approximately 200 m above the village. The project is in an area of low relief with broad rolling hills in Central Anatolia between Ankara and Kayseri.

SRK Consulting (US), Inc. (SRK) issued a conceptual report titled "Himmetdede Heap Leach Pad Conceptual Design" (SRK, 2011). SRK considered available area and capital costs in selecting the heap leach pad that was most suitable for the site, and SRK selected an on-off heap leach pad (OHLP) from a footprint and cost perspective. Three potential heap leach sites were considered, and based on comments from Koza, SRK developed an OHLP design for the option that was closest to the mine and highway.

Samples of 32 mm and 9.5 mm nominal material ( $P_{80}$ ) for the Project have been provided to McClelland Laboratories, Inc. (McClelland) from Sparks, Nevada for metallurgical testing. As of the effective cut-off date for the Himmetdede Project (Project), SRK did not have any site-specific ore material that could be tested for material properties. Therefore, SRK made assumptions on material properties to develop the HLP designs, and assumed conservative lift heights in the order of 4 m, which will be confirmed in subsequent stages of design.

The objective of this study is to provide a prefeasibility (PFS) level design for a Heap Leach Pad (HLP) at the Himmetdede Project (Project) that could contain approximately 29 million tonnes (Mt) of ore, corresponding to eight years of mining, at a rate of 11,000 tonnes per day (11 ktpd) and operating 360 days per year. SRK designed the OHLP with a composite liner configuration to minimize leach solution losses, along with an Overliner system comprised of free draining gravel and piping system to allow for timely solution recovery. Depending on the spent ore rinsing or detoxification method used, the ore may still need to be placed within an area that has a barrier system of some type and any recovered solution collected. Therefore, after the leaching has been completed and rinsing and draindown cycle of the ore has been completed, the spent ore would be removed from the OHLP and transported to the Spent Ore Facility (SOF). SRK's design allowed for a conveyor stacking and front end loader reclaim system.

Based on the cost assumptions developed in the report, SRK developed the Life of Mine (LoM) capital and operating cost estimate to be \$133.5 MUSD. The cost estimate includes a contingency (usually 35%) which is typical for a PFS level design.

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# **Disclaimer**

# Heap Leach Pad Prefeasibility Level Engineering Design - Himmetdede Project

The following document for the Himmetdede Gold Heap Leach Project has been prepared by the staff of SRK Consulting (US), Inc. (SRK) for Koza Altin Isletmeleri A.S. (Koza).

The report, figures, drawings and appendices for the Himmetdede Heap Leach Project facilities are presented within the limits described by Koza, and were prepared in accordance with generally accepted professional engineering principles and practice.

The opinions expressed in this Report have been based on the information supplied to SRK by Koza and are provided in response to a specific request from Koza to do so. SRK has exercised all due care in reviewing the supplied information. Whilst SRK has compared key supplied data with expected values, the accuracy of the results and conclusions from the review are entirely reliant on the accuracy and completeness of the supplied data. SRK does not accept responsibility for any errors or omissions in the supplied information and does not accept any consequential liability arising from commercial decisions or actions resulting from them.

This document was prepared for the exclusive use of Koza and their staff and consultants for specific application of the Himmetdede Heap Leach Project. The findings, recommendations, and conclusions for this design are based on results of engineering analysis and review of information from SRK and Koza files, combined with SRK's experience on similar projects and understanding of the Project as stated in this document. If changes occur, the design documents presented herein will not be considered valid unless the changes are reviewed by SRK and the appropriate conclusions of this document are modified and verified in writing.

Neither SRK nor any of the authors of this report have any material present or contingent interest in the outcome of this report, nor do they have any pecuniary or other interest that could be reasonably regarded as being capable of affecting their independence or that of SRK. SRK's fee for completing this Report is based on its normal professional daily rates plus reimbursement of incidental expenses. The payment of that professional fee is not contingent upon the outcome of the Report.

# **List of Abbreviations**

Abbreviation	Unit or Term
ATP	ATP İnşaat ve Ticaret A.Ş
BMP	Best Management Practices
°C	Degrees Celsius
CAPEX	Capital Expenditure
cm/sec	Centimeter per second
0	Degree (degrees)
DHLP	Dedicated Heap Leach Pad
Dwg	Drawing
EPCM	Engineering, Procurement and Construction Management
Fig	Figure
HDPE	High Density Polyethylene
H:1V	Horizontal to Vertical
HLP	Heap leach pad
HTVL	High tension voltage line
hr	Hour
ID	Inside Diameter
IFC	Issued for Construction
ILHLP	Interlift Heap Leach Pad
KIH	Koza-İpek Holding A.Ş.
kg/m <sup>3</sup>	kilogram per cubic meter
km	Kilometer
Koza	Koza Altin Isletmeleri A.S.
kPa	Kilopascals
ktpd	Thousand tonnes per day
LCRS	Leak Collection Recovery System
L/hr/m <sup>2</sup>	Litres per hour per square meter
LDS	Leak Detection System
LoM	Life of Mine
m	Meter
Mm <sup>2</sup>	Million square meters
mm	Millimeter
MCE	Maximum Credible Earthquake
Mt	Million tonnes
NMAS	Normandy Madencilik A.Ş.
OHLP	On-off heap leach pad
OPEX	Operating Expenditure

PCPE	Perforated, corrugated, single-walled polyethylene
PGA	Peak ground acceleration
PLS	Pregnant Leachate Solution
psi	Pounds Per Square Inch
SOF	Spent Ore Facility
SRK	SRK Consulting, Inc.
tpd	tonnes per day
USGS	United States Geological Survey
UTM	Universal Transverse Mercator
$m^3$	cubic meter

The metric system has been used throughout this report unless otherwise stated.

## 1 Introduction

In March 2005, Koza-İpek Holding A.Ş. (KIH) and ATP İnşaat ve Ticaret A.Ş, (ATP), an indirectly owned subsidiary of KIH, and together with KIH, collectively the "Koza Group", acquired 40% and 60%, respectively, of the shares of Normandy Madencilik A.Ş. (NMAS) from Newmont Mining Corporation Limited (Newmont). Subsequent to the Koza Group's acquisition of NMAS, NMAS's name was changed to its current name, Koza Altın İşletmeleri A.Ş. (Koza). In 2005, Koza became the first Turkish company in the history of the Republic of Turkey to realize gold production within Turkey. Koza's mission is to carry out gold mining operations using the best available technology while displaying the highest environmental performance.

The Himmetdede Project is located in central Anatolia approximately 35 kilometers (km) northwest of Kayseri, along road D260, and adjacent to the village of Himmetdede.

On August 15, 2011, SRK Consulting (US), Inc. (SRK) issued a conceptual report titled "Himmetdede Heap Leach Pad Conceptual Design" (SRK, 2011). SRK considered available area and capital costs in selecting the heap leach pad that was most suitable for the site and SRK selected an on-off heap leach pad from a footprint and cost perspective. Three potential heap leach sites were considered, and based on comments from Koza, SRK developed an on-off heap leach pad (OHLP) design for the option that was closest to the mine and highway.

## 1.1 Scope of Work

The objective of this study is to provide a prefeasibility (PFS) level design for a Heap Leach Pad (HLP) at the Himmetdede Project (Project) that could contain approximately 29 million tonnes (Mt) of ore, corresponding to eight years of mining, at a rate of 11,000 tonnes per day (11 ktpd) and operating 360 days per year. The scope of work performed by SRK for this report is outlined in the proposal for the PFS design for the Himmetdede OHLP (SRK, 2011a), and generally includes the following:

- Develop a PFS level OHLP facility design, based on design criteria that have been developed with the client;
- Using the available site topographic contours provided by Koza, develop an OHLP option and layout;
- Develop capital and operating cost estimates; and
- Provide a basis for detailed design and site characterization study recommendations.

# 1.2 Report Layout

The layout of the Design Report is as follows:

- Section 1 presents a brief introduction to the Project and an outline of the report;
- Section 2 provides a Project description and design criteria for the Project;
- Section 3 discusses the geotechnical, geological, and hydrogeological conditions in the Project area;
- Section 4 discusses the OHLP siting and recommendations;
- Sections 5, 6 and 7 describes the OHLP, Solution Pond and Spent Ore Facility (SOF) Design, respectively;
- Section 8 discusses material handling;
- Section 9 presents costing considerations;

- Section 10 presents the OHLP capital and operating cost estimate;
- · Section 11 discusses environmental considerations; and
- · Section 12 presents conclusions and recommendations for the Project.

# 1.3 Project Team

The following staff members from SRK were involved with this study:

- Dr. Dirk van Zyl: Senior Technical Reviewer;
- Terry Mandziak: Principal Engineer and Project Manager Heap Leach Design;
- John Danio: Principal Associate Engineer;
- · Eric Johnson: Senior Engineer; and
- Brian Ward: Staff Civil Engineering Consultant.

The SRK team was ably assisted by Koza staff.

# 2 Project Description and Design Criteria

#### 2.1 General Site Location

The Himmetdede Project is located in central Anatolia approximately 35 km northwest of Kayseri, along road D260, and adjacent to the village of Himmetdede, as shown in Drawing 1.

The Project is located at an elevation of approximately 1,200 meters (m) and the project rises approximately 200 m above the village. The project is in an area of low relief with broad rolling hills.

# 2.2 Design Criteria

Appendix A presents typical design criteria and parameters that were utilized for the OHLP design, generally based on assumed values or previous experience on other projects. As part of subsequent levels of design, SRK recommends that a set of site specific design criteria should be developed.

#### 2.2.1 Average Annual Precipitation and Evaporation

The Himmetdede Project is located in Central Anatolia between Ankara and Kayseri. This is a region with a continental climate, i.e. cold, harsh winters and dry summers with moderate to hot temperatures. Average temperatures range from 0°C in January to 22°C in July and August. The maximum temperatures may reach 40°C in the summer. Local rainfall data indicates average annual precipitation is 350 to 400 millimeters (mm), which falls as rain during the summer months and snow during the winter months.

Climatological data is presented in Appendix A, and summarized below.

Table 2-1: Climatological Data

Month	Average Precipitation (mm)	Average Evaporation (mm)	Average Minimum Temperature (°C)	Average Maximum Temperature (°C)
Jan	31.7	0	-6.9	3.8
Feb	33	0	-5.3	5.7
Mar	41.1	0.6	-1.5	11.5
Apr	56.3	58	3.5	17.6
May	57.4	124.5	6.8	22.1
Jun	36	172	9.7	26.7
Jul	13.1	216.5	12.1	30.5
Aug	6.5	201.6	11.5	30.6
Sep	11.4	140.4	7.4	26.5
Oct	33.3	68.9	3.8	20.2
Nov	38.2	1.7	-1.1	12.2
Dec	39.4	0	-4.7	5.8
Annual	397.4	984.2	2.9	17.8

#### 2.2.2 Storm Events

SRK did not have site specific storm event data. For the purpose of the PFS, a 100-year, 24-hour storm event of 100 mm was assumed.

### 2.2.3 Topography

The site topography was provided by Koza and was generated on 5 m contour intervals.

# 3 Site Geotechnical, Geological and Hydrogeological Setting

#### 3.1 Site Reconnaissance

As part of the conceptual scope, Mr. Terry Mandziak of SRK and Mr. Serhan Umurhan of Koza visited the Project site. Two site visits were made as part of the conceptual level field reconnaissance for the OHLP design: the first on March 17 and 18, 2011; and the second on May 12, 2011.

As part of the PFS scope, Mr. Terry Mandziak of SRK was onsite during part of the Phase I geotechnical program, on October 17 and 18, 2011. This is discussed in more detail in Section 3.7.

## 3.2 Geology

The hilly parts of the Himmetdede region consist of Lutetian Formation marbles and metamorphic rocks. Marbles are course- to fine-grained, crystalline and sometimes the presence of breccia zones is observed. The rocks are grey-white, pink or purple. The thickness of these marbles is about 20 to 30 cm but at places they are thicker and more massive. In some places marble levels are interbedded with chlorite schists. The Lutetian and Neogene aged geological formations are found within the OHLP and SOF areas.

The Miocen aged Neogen formation is composed of the following:

- Lacustrine Limestone; the lacustrine limestone which are generally flaggy (5 to 10 cm in thickness) or travertine in structure or passing from one to another laterally and vertically. These limestone layers overlie the marbles and crystalline schist levels;
- Tuffs; tuffs are generally 1 m thick and homogeneous in structure. Towards west they contain clay or sandy clayey beds. The color of these tuffs is generally white, pink and rarely black. These tuffs are used as building material all over the area; and
- Sandy, clayey, calcareous tuffs; these heterogeneous lake deposits generally are seen at the east and northeast of Himmetdede. At the project site, calcareous tuffs were only seen in Test Pits 24, 28 and 29.

# 3.3 Geologic Hazards

No geologic hazards investigation or characterization was performed as part of this study, and surface water data was limited to observations made as part of the field reconnaissance.

# 3.4 Seismicity

SRK undertook a review of the United States Geological Services (USGS) Seismic Hazard Setting Map (USGS, 2008), which provides an estimate of the peak ground acceleration (PGA) with a 10% chance of exceedance in 50 years, and is presented in Figure 1. Ground acceleration resulting from the Maximum Credible Earthquake (MCE) was estimated from the map as approximately 0.20 g.

# 3.5 Hydrogeology

No hydrogeologic investigation or characterization was performed as part of this study, and groundwater data was limited to a desktop review of existing information.

#### 3.6 Surface Water

No surface water investigation or characterization was performed as part of this study, and surface water data was limited to observations made as part of the field reconnaissance. As the OHLP area is located near the top of the surface water basin area, SRK expects a relatively limited upgradient contributing area.

# 3.7 Geotechnical Investigation

#### 3.7.1 Heap Leach Pad and Spent Ore Facility Foundation

A Phase I geotechnical program was performed by Fugro Sial Geosciences Consulting & Engineering Ltd. (Fugro) from October 17 through 19, 2011. The scope performed by Fugro consisted of the following items:

- Collect information about previous studies done on the project site and maps related with the study area;
- Excavate 35 Test Pits within the Mine, OHLP, SOF and Waste Rock Facilities, in order to determine the type, thickness, sequence, conditions and properties of the ground materials;
- · Take representative bulk samples from the Test Pits for the laboratory works; and
- Perform geotechnical laboratory tests on the bulk samples from Test Pits.

The results of the Phase I program are presented in Appendix B.

#### 3.7.2 Ore Characterization

Samples of 32 mm and 9.5 mm nominal material ( $P_{80}$ ) for the Project have been provided to McClelland Laboratories, Inc. (McClelland) from Sparks, Nevada for metallurgical testing. As of the effective cut-off date for the Project, SRK did not have any site-specific ore material that could be tested for material properties. Therefore, SRK made assumptions on material properties to develop the HLP designs, which are discussed in subsequent sections and will need to be confirmed as part of the next stage of design.

Core photographs from a typical drillhole showed a high clay content, and the highly fractured appearance suggests that the ore will require agglomeration. SRK assumed conservative lift heights in the order of 4 m, which will be confirmed in subsequent stages of design.

#### 3.7.3 Borrow Materials

Limited geotechnical investigations were performed as part of this study, and geotechnical data was limited to the Phase I Field Program and a desktop review of existing information. Therefore, SRK made assumptions on material properties to develop the OHLP design, which are discussed below and will need to be confirmed as part of the next stage of design.

#### **Compacted Fill**

From the Phase I Geotechnical Program, SRK assumed that local cut materials would be available for fill material required to meet the design grades. After removal of the topsoil and other deleterious materials, proper moisture conditioning and compactive efforts of excess cut could be used for Compacted Fill.

#### Soil Liner

The Phase I Geotechnical Program identified a potential Soil liner fill borrow source. Three permeability tests were performed, each of which achieved a permeability less than 1x10<sup>-6</sup> cm/sec.

#### **Overliner**

While crushed ore is generally preferred as a source for Overliner material, the core photographs and SRK experience suggest that the material will not be durable enough to meet the Overliner requirements.

Koza had identified an offsite granite source located about 10 km from the OHLP site. Additional characterization work will need to be performed to verify that the physical and chemical durability meet the project requirements.

#### 3.7.4 Blasting Rock Removal

During the geotechnical investigation of the heap leach pad site it was determined that a layer of rock was located directly under the proposed site that will require blasting for removal. Based on the following sources of information a rock surface was established:

- Phase 1 Geotechnical Report;
- · Phase 2 Geotechnical Report; and
- Site Investigations by SRK Staff.

Required blasting volumes were then calculated based on the following criteria:

- · Minimum blasting depth of 1m; and
- Overblasting depth of 0.5 meters to ensure a competent base for compacted fill.

A final blasting volume of approximately 122,700 m<sup>3</sup> was calculated for the base OHLP design.

Quantities were estimated assuming 5% of the compacted fill volume of the ponds.

# 4 Heap Leach Pad Siting

As part of the Conceptual OHLP Design (SRK, 2011) SRK evaluated site-specific criteria, such as ore characteristics, available area, and costs, considering the following OHLP options for the Himmetdede Project:

- Dedicated Heap Leach Pad (DHLP);
- · Heap Leach Pad with Interlift liners (ILHLP); and
- OHLP with conveyor stacking and front end loader reclaim system.

Based on an inspection of the core and core photos, SRK had assumed that the ore would require agglomeration and that ore lift heights would be in the order of 4 m. Therefore, for the purpose of the conceptual study, SRK selected an OHLP from a footprint and cost perspective. This assumption was continued as part of the PFS design, and will need to be confirmed in subsequent stages of design.

During the conceptual study site visit, three potential OHLP sites were identified in the area around the Himmetdede open pit mine, and include the following information:

- Option 1: Area northwest of the town of Tathasankuyusu, north of the mine and west of railroad tracks;
- · Option 2: Area northwest of the town of Himmetdede; and
- Option 3: Area west of the town of Duver.

SRK developed a set of ranking criteria that can be used in a semi-quantitative evaluation system. Based on the siting study performed, Option 2 was selected as the preferred location for the OHLP, and was used in the PFS.

# 5 On Off Heap Leach Pad Design

The OHLP was designed based on the following operating requirements:

- 11 ktpd crushed and agglomerated ore;
- 70-day leach cycle;
- Maximum stacked ore height of 4 m;
- Nominal or angle of repose of 1.3 H:1V;
- Average inplace ore dry density of 1,600 kilograms per cubic meter (kg/m³); and
- 100-year, 24-hour Design Storm Event.

# 5.1 On Off Heap Leach Pad Size and Configuration

For each the OHLP design and configurations considered, the maximum layout was generally constrained by the following:

- To the north and east, by a 500 m buffer between the OHLP limits and the railroad line;
- To the south, by a 500 m buffer between the OHLP limits and the highway;
- To the east, by a 1000 m buffer between the OHLP limits and the town of Duver;
- Twin high tension voltage lines (HTVL) that run through the center of the Project; and
- A buried natural gas line that runs through the center of the project.

Physical constraints and buffers were discussed in the conceptual report (SRK, 2011), along with the basis for recommending an OHLP.

# 5.2 On Off Heap Leach Pad Liner System

#### 5.2.1 Composite Liner System

The composite liner configuration for the Project was selected to minimize leach solution losses which could be mainly due to defects in geomembrane manufacturing process or damage from the installation and construction.

SRK developed the PFS design to have the following configuration (from bottom to top):

- Prepared subgrade;
- 300 mm thick low permeability Soil Liner compacted to achieve a minimum permeability of 1x10<sup>-6</sup> centimeters per second (cm/sec); and
- 2.0 mm High Density Polyethylene (HDPE) geomembrane synthetic liner.

No Leak Detection System (LDS) has been proposed in the OHLP PFS design, as the above liner configuration was considered to minimize the risk of operational seepage losses.

### 5.3 On Off Heap Leach Pad Solution Collection System

SRK has defined the Solution Collection system for the OHLP as the Overliner and the Solution Collection Piping. The design of these two components is discussed in the following sections.

#### 5.3.1 On Off HLP Overliner Design Considerations

The Overliner for the OHLP was designed to account for the following:

- Construction and Static Loading. Static loading conditions refer to the ore placed at a depth of 4 m and correspond to a normal load of approximately 60 kilopascals (kPa) assuming an ore density of 1,600 kg/m³. This is a relatively minor static load and the Overliner thickness should be based on construction placement limitations and should not be less than 600 mm (i.e., there is sufficient Overliner to prevent construction equipment damage to the geomembrane);
- Dynamic Loading. Ore will be placed onto the Overliner using one of either:
  - A track mounted ore stacking system and removed using a track mounted reclaiming system. Data provided from the manufacturer of the system indicate a maximum track/soil contact pressure of approximately 200 kPa (29 pounds per square inch (psi)) when a loaded tripper is working directly over a single track. For the dynamic loading conditions, SRK developed the design with a minimum of 600 mm of Overliner should be placed over the Geomembrane; and
  - Grasshoppers and removal with loaders. Ground contact pressures of wheel mounted equipment are significantly higher than track mounted equipment, and can be in the range of 550 to 690 kPa (80 to 100 psi). For this condition, SRK considered a minimum Overliner thickness of 900 mm; and
- Overliner Performance. A key issue that impacts the performance of an OHLP is the long term
  performance of the Overliner material. The performance of the Overliner is particularly sensitive
  to a low permeability boundary that can be created at the Ore and Overliner interface from one,
  or a combination of, the following:
  - During the spent ore reclaiming process, the tracks from the reclaim stacker or wheels from the loader can compact the surface and ripping this interface is a common means of improving the permeability. The ripping process usually mixes the spent ore and the Overliner surface material, creating a layer or interface that has a lower permeability;
  - Breakdown of particles from the repeated trafficking of the stacking and reclaiming equipment on the Overliner surface, as well as any other mechanical equipment that may be working on the Overliner surface; and
  - The migration of fines from the ore through the leaching process.

In addition to increasing the solution recovery time, decreasing the permeability of the Overliner at this interface would increase the thickness of the saturated zone in the ore and could adversely impact the stability of the stacked ore.

While SRK has made recommendations for the thickness of the Overliner material based on static, dynamic, and construction loading, SRK has designed the OHLP so that additional 300 mm of Overliner material can be placed to allow for Overliner material being removed via the reclaimer (and replaced by regular construction methods) every two years in order to maintain an efficient and effective permeability at the Ore / Overliner interface.

For the material handling system proposed, SRK recommended an Overliner thickness of 1200 mm to account for the increase static and dynamic loading.

#### 5.3.2 On Off Heap Leach Pad Solution Collection Piping

SRK would propose a solution collection system that consists of a solution collection piping network placed directly on the geomembrane liner and buried within the Overliner material. The solution collection piping system was designed to meet the following criteria:

- Generate a maximum head of 300 mm on the Geomembrane;
- Convey the design flow capacity of the pipe, considering the maximum length of each pipe, the
  pipe spacing (calculated from the maximum head calculations) and the design solution
  application rate of 10 L/hr/m<sup>2</sup>;
- Pipe loading, considering both static and cyclic loading; and

· Minimize thermal expansion.

SRK would propose the following Solution Collection configuration for the OHLP:

- Tertiary solution collection pipes consisting of 100 mm inside diameter (ID) perforated, corrugated, single-walled polyethylene (PCPE), placed directly on the geomembrane at a 1% grade;
- Secondary solution collection pipes, such as 251 mm ID double walled PCPE or 380 mm ID double walled PCPE. The pipes would be placed in Solution Trench with a minimum flow line grade of about 1%; and
- At the edge of the OHLP, solutions from the Secondary HDPE pipes would then be transferred into an open trench or an HDPE SDR 17 / PN 10 Primary solution collection pipe to the Solution Ponds.

# 5.4 On Off Heap Leach Pad Closure

SRK developed the PFS cost estimate, assuming the following reclamation activities for the OHLP area:

- Spent ore from the OHLP, including Overliner material, would be removed after the last leach cycle and placed in the SOF;
- The OHLP geomembrane would be cut, removed and placed within in the Solution Pond footprint by the general labor crew; and
- The OHLP area would be regraded to blend with natural contours by the general labor crew and covered with 300 mm of topsoil.

# 6 Solution Pond Design

# 6.1 Solution Pond Sizing and Configuration

Typically, a site specific water balance is developed in order to adequately size the Solution Ponds as well as to identify any makeup water needs or excess water treatment requirements. This water balance model is typically developed on a monthly basis, and reflects seasonal variations in precipitation, evaporation and operating parameters (leach cycle, irrigation rates, etc.). Using the project design criteria, the water balance considers inflow (such as precipitation and solution application), as well as outflows (such as evaporation from both pad and pond areas and losses due to ore uptake), as well as changes in the heap leaching parameters (ore loading, losses due to ore uptake, etc.) on a monthly basis.

A site specific water balance was constructed using an Excel™ spreadsheet developed specifically for the Himmetdede Project. A general discussion of the water balance components and results appears below in section 6.1.1. An electronic and hard copy of the water balance spreadsheet along with a detailed column by column description of the water balance calculations appears in Appendix C.

#### 6.1.1 Water Balance Calculations and Results

The Solution Pond system was developed utilizing the following mass balance systems as described below and presented in Appendix C:

- Mass Balance 1 (OHLP Active Leaching Cells): Ore enters into the system from the mine where
  it is agglomerated and stacked on to the OHLP. Barren Solution enters Mass Balance 1 from
  the Process Plant, and is either used to wet the ore in the Agglomeration drum or is used to
  leach the ore. Pregnant solution is collected and reports to the Pregnant Pond, where gold is
  recovered and Barren Solution is re-introduced into the system. Mass Balance 1 is then used to
  size the Pregnant Pond;
- Mass Balance 2 (OHLP Rinsing Cells): At the end of leaching, ore from mass Balance 1 is rinsed
  with freshwater for 14 days to remove cyanide solution. The Rinse Solution then reports to a
  Rinse Pond, where it is pumped to the Process Plant and introduced into Mass Balance 1 as
  makeup water. Mass Balance 2 is then used to size the Rinse Pond, and any excess Rinse
  solution that the Process Plant cannot accept must be treated and discharged; and
- The Storm Pond sizing is independent of the Mass Balances, and based on the design storm event depth, multiplied by the total OHLP and Pond areas.

The results of the water balance for the active leaching cells (Mass Balance 1) indicate that the system is a net user, or requires makeup water addition. However, the rinse cells (Mass Balance 2) are a net water producer, or generates excess water. SRK investigated combining the two systems, performing a combined mass balance between excess water created annually by the rinsing system against the amount of make-up water by altering the application rate of the freshwater rinse solution. The effect of this overall water balance calculated a maximum rinse application rate of 1.50 l/hr/m² in order to achieve a net balance on an annual basis between Mass Balance 1 and Mass Balance 2. In other words, if the rinse application rate exceeds 1.50 l/hr/m², then water treatment of the rinse solution will be required. If the application rate is less than this amount then make-up water will be required for the process plant.

Table 6-1 shows the sizing for the Pregnant, Rinse and Storm Ponds as calculated from the water balance calculations.

Table 6-1: Pond Sizing

Output	Pregnant	Rinse	Storm
Max Pond volume (m <sup>3</sup> )	7,500	38,000	27,000

#### 6.1.2 Pond Configuration

The Ponds were designed based on the following operating requirements:

- 2.0 H:1V sideslopes; and
- 5 m maximum excavation depth.

#### 6.1.3 Pond Liner System

There are two types of pond liner systems that have been designed for the Project, and they are as follows:

- Ponds that will be containing leach solution and having a head on the liner system on a regular basis have been designed with a double liner system and compacted low permeability soil. The double liner system will have a high permeability drainage layer that will allow solution that may leak from the upper Secondary geomembrane to be collected and removed via pumping, to minimize the head acting on the lower Primary geomembrane; and
- Ponds that will be containing solution on an infrequent basis have been designed with a single liner system.

#### 6.1.4 Pregnant and Rinse Ponds

As the Pregnant and Rinse Ponds will have solution stored on a regular basis (and generate a corresponding head on the geomembrane), SRK proposes a design with the following double liner systems (from the foundation upward):

- · Prepared subgrade;
- 300 mm thick low permeability Soil Liner compacted to achieve a maximum permeability of 1x10<sup>-6</sup> cm/sec;
- 1.5 mil HDPE geomembrane secondary liner;
- · Geonet drainage layer; and
- 1.5 mil HDPE geomembrane primary liner.

The HDPE geomembrane was selected due to its superior UV resistance as compared to other geomembrane materials.

The geonet will function as part of the Leak Collection Recovery System (LCRS). This layer has been added to prevent the excessive head being generated on the lower (Secondary) geomembrane should a defect or leak occur in the upper (Primary) geomembrane. The LCRS will have a highly permeable free draining geonet material, and any fugitive solution will be collected in the LCRS and flow via gravity to the LCRS sump where it can be collected and pumped back into the pond system. Beneath the geonet is an HDPE primary geomembrane, which will be installed on a layer of compacted low permeability soil liner material.

#### 6.1.5 Stormwater Pond

As the Stormwater Pond will be used on an infrequent basis (during extreme storm events when the capacity of the other ponds has been exceeded), SRK would propose that the Stormwater Pond be designed with the following single liner configuration (from the foundation upward):

- · Prepared subgrade;
- 300 mm thick low permeability Soil Liner compacted to achieve a minimum permeability of 1x10<sup>-6</sup> cm/sec; and
- 1.5 mil HDPE geomembrane liner.

The HDPE geomembrane was selected due to its superior UV resistance as compared to other geomembrane materials.

#### 6.2 Solution Ponds Closure

SRK developed the PFS cost estimate, assuming the following reclamation activities for the Solution Pond areas:

- The Solution Pond geomembrane would be cut, removed and placed within in the Solution Pond footprint by the general labor crew; and
- The Solution Pond areas would be regraded to blend with natural contours by the general labor crew and covered with 300 mm of topsoil.

# 7 Spent Ore Facility Design

Depending on the spent ore rinsing or detoxification method used, the ore may still need to be placed within an area that has a barrier system of some type and any recovered solution collected. Therefore, after the leaching has been completed and rinsing and draindown cycle of the ore has been completed, the spent ore would be removed from the OHLP and transported to the SOF. The difference between the SOF and the OHLP is that the SOF is typically designed for stability (which may include an elevated phreatic surface), not percolation and leaching of the ore.

The SOF was designed based on the following operating requirements:

- Total storage volume of 29.2 Mt, which corresponds to 8 years of storage operating at 360 days per year;
- Maximum spent ore height of 30 m (typical);
- Average inplace ore dry density of 1,600 kg/m<sup>3</sup>; and
- Located within 3 km of the OHLP.

## 7.1 Spent Ore Facility Constraints

For each the SOF design and configurations considered, the maximum layout was generally constrained by the following:

- A 30 m maximum height;
- To the north and east, by a 500 m buffer between the OHLP limits and the railroad line;
- To the south, by a 500 m buffer between the OHLP limits and the highway;
- To the east, by a 1000 m buffer between the OHLP limits and the town of Duver;
- Twin HTVL that run through the center of the project; and
- A buried natural gas line that runs through the center of the project.

# 7.2 Spent Ore Facility Liner System

The liner configuration for the SOF was selected to minimize leach solution losses which could be mainly due to defects in geomembrane manufacturing process or damage from the installation and construction. However, consideration was given to the fact that spent ore was being permanently placed at a lower water content, and ore would not be subjected to secondary leaching. Therefore, SRK developed the PFS design of the SOF to have the following configuration (from bottom to top):

- Prepared subgrade;
- 300 mm thick low permeability Soil Liner compacted to achieve a minimum permeability of 1x10<sup>-6</sup> cm/sec; and
- 1.5 mm High Density Polyethylene (HDPE) geomembrane synthetic liner.

No LDS has been proposed in the SOF PFS design.

# 7.3 Spent Ore Facility Solution Collection System

SRK has defined the Solution Collection system for the SOF as the Overliner and the Solution Collection Piping. The design of these two components is discussed in the following sections.

#### 7.3.1 SOF Overliner Design Considerations

The Overliner for the SOF was designed to account for the following:

- Construction and Static Loading. Static loading conditions refer to the ore placed at a maximum depth of 30 m and correspond to a normal load of approximately 440 kPa assuming an ore density of 1,600 kg/m³. This is a relatively minor static load and the Overliner thickness should be based on construction placement limitations and should not be less than 600 mm (i.e., there is sufficient Overliner to prevent construction equipment damage to the geomembrane); and
- Dynamic Loading. Ore will be placed onto the Overliner using one of either:
  - A wheel mounted portable conveying system. For the dynamic loading conditions, SRK developed the design with a minimum of 600 mm of Overliner should be placed over the Geomembrane; and
  - Trucks. Ground contact pressures of wheel mounted equipment are significantly higher than track mounted equipment, and can be in the range of 550 to 690 kPa (80 to 100 psi).
     For this condition, SRK considered a minimum Overliner thickness of 900 mm.

For the material handling system proposed, SRK recommends an SOF Overliner thickness of 900 mm to account for the static and dynamic loading.

#### 7.3.2 On Off Heap Leach Pad Solution Collection Piping

As the spent ore material is not under active leach, SRK assumed that a solution collection system would not be required.

## 7.4 Spent Ore Facility Phased Design

In order to reduce the problems associated with storm water management, including ponding and potential flooding on the upstream face, along with deferring capital, the SOF could be constructed in phases. The SOF construction should start from the upgradient side in the uppermost portion of the proposed area, with downstream sections added in subsequent phases. This plan would allow surface water to be diverted around the SOF, allows the construction of the SOF to be completed in stages to reduce costs to later periods in the project's life, and while achieving a stable design.

In order to accommodate a phased approach, SRK developed a flexible design that allows for spent ore to be taken off the conveyor belt line at two points:

- The end of the discharge conveyor from the OHLP; and
- At the end of the first overland conveyor that parallels the SOF north-west corner.

#### 7.5 SOF Closure

SRK developed the PFS cost estimate, assuming the following reclamation activities for the SOF areas:

- All side slopes of the SOF be final graded to a grade not to exceed 3 H:1V;
- The surface of the SOF regraded with a 1% cross slope;
- All side slopes be armoured with a 500 mm thickness of D<sub>50</sub> = 250 mm rock; and
- The crest be covered with 300 mm of topsoil, and revegetated.

# 8 Material Handling and Equipment Design Considerations

The conveying system design assumed for OHLP loading and unloading was developed to provide robust and flexible operations. All the conveyors proposed have been designed to work as a team and share the following common characteristics:

- 750 tph capacity at 115 m/min belt speed; and
- 915 mm belt width.

Information provided by material handling vendors is presented in Appendix D.

## 8.1 Pad Loading System

Ore will be transferred from the crushing and agglomerating areas to the HLP area using a fresh ore fixed conveyer. The fixed conveyor has rail road rails on either side to accommodate a rail tripper. The rail tripper diverts the ore from the fixed conveyor and discharges the material onto the first of a series of grasshopper conveyors. The grasshoppers have been designed at 35 m lengths to provide maximum length consistent with stability and mobility, with the power and control logic interlocked. Each grasshopper has rubber tires for easy movement and has a feed hopper designed to receive material from another grasshopper. Based on the OHLP dimensions, ten grasshoppers will be needed to traverse the length of the OHLP.

The last grasshopper in the series then discharges onto the horizontal feed conveyor, which in turn discharges onto the horizontal conveyor. The horizontal feed conveyor and the horizontal conveyor are designed to work in concert with the conventional stacker. These two belts provide a flexible feed location for the conventional stacker so that 35 m of pad can be stacked before a grasshopper needs to be removed from the system. The horizontal feed conveyor is at an angle of 90 degrees to the other belts in the system and elevates the ore from the discharge elevation of the grasshopper to the higher elevation of the horizontal conveyor. The horizontal conveyors discharge elevation is fixed to the conventional stacker feed hopper.

The horizontal conveyor discharges to the conventional stacker which is the last conveyor in the system. The conventional stacker places the ore onto the OHLP. The conventional stacker is 36.5 m long with an extendable 6 m stinger belt for a total length of 42.5 m. The 36.5 m stacker operates over a 180 degree arc, and this geometry then defines the cell's typical width of 60 m with some overlap on the sides. The stacker places the ore in the OHLP at the design height, with the stacking height hydraulically adjusted, so that the ore depth can be varied should ore characteristics change. The traverse speed, or the rate at which the stacker moves along its arc, is hydraulically controlled so that operator can monitor the characteristics of the ore. Probes can be mounted on the end to automatically move the discharge position. The stinger belt, 6 m long, is set up to retract in 1 m increments so that the ore is stacked in thin layers with a reasonably smooth top surface. Once the stinger belt is completely retracted, the stacker is then moved up 6 m and operations resume.

# 8.2 Spent Ore Material Handling Options

Spent ore from the OHLP will be off-loaded using a front end loader that places spent ore from the pad onto a mobile conveyor hopper. The hopper dimensions are coordinated with the loader bucket width and normal dump height. The mobile conveyor hopper has a flight-type feeder in the hopper

bottom to positively move the spent ore onto an integral elevating belt that discharges onto the first of a series of eleven grasshopper conveyors. The last of the grasshoppers discharges the spent ore into the rail hopper which is mounted over the fixed conveyer located on the upgradient side of the OHLP.

SRK considered five options to place the spent ore from the leach pad onto the designed area, as follows:

- Overland Conveyors along NW side of pad with 3 grasshoppers, for elevation, and scrapers for final placement;
- · Overland Conveyors and 20 grasshoppers with radial stacker;
- Overland Conveyors with 1 radial stacker, loader and trucks;
- No overland Conveyor(s) with 1 radial stacker, loader and trucks; and
- Overland Conveyor(s) to mid-point then loader and trucks, extend overland in year 3. A radial stacker would be used to provide stockpile capacity.

For PFS costing purposes, SRK costed the final option, and assumed that spent ore would be transferred to a 35 m long radial stacker at the end of the fixed conveyor, placing the spent ore material into a stockpile, with an estimated volume of 13 kt. A front end loader would load spent ore from the stockpile onto one of two trucks for transport to the SOF dump face. The face would be built in an up gradient manner, keying off of the internal berm, and use a tracked dozer for final maintenance. The top surface of the spent ore would be graded to direct any runoff to the northwest side where it would be added to any runoff from the plant or pad.

# 8.3 Material Handling Closure Considerations

SRK developed the PFS cost estimate, assuming the following reclamation activities for the Material handling corridors:

- All roadways connecting the OHLP and SOF be ripped to a depth of 150 mm, regraded to match the natural terrain, and revegetated;
- Surface infrastructure (piping and conveyors) removed; and
- Surface infrastructure areas regraded to blend with natural contours, covered with topsoil and revegetated.

# 9 Costing

## 9.1 Purpose and Scope of the Estimate

The PFS capital cost estimate for the Project provides the estimated cost to construct the OHLP proper and is to be used as part of an internal decision making process, document the design and assumptions, and provide recommendations to reduce the costing uncertainty. As no geotechnical investigations were performed as part of this study, SRK developed the PFS design and costing using the following assumptions and key observations made during the field reconnaissance:

- Excess cut material from within the HLP footprint is suitable for fill;
- Borrow materials (low permeability clays) meeting specified requirements are readily available onsite; and
- Overliner material will be from an offsite source.

#### 9.2 Units of Measurement

All units are metric, unless otherwise noted.

## 9.3 Exchange Rates

Costs are provided in 2011 US dollars.

#### 9.4 Take Off Procedures, Allowances and Factors

All earthworks quantities are taken off neat in place, with no allowance for swell or compaction of materials. Industry-standard allowances for swell and compaction are incorporated into the unit cost assumptions.

# 9.5 Contingency

Contingency is defined as an allowance to cover unforeseeable items within the scope of this capital cost estimate. Contingency can arise due to currently undefined items of work or equipment, lack of site-specific geotechnical data, uncertainty in the estimated quantities and unit prices for labor and equipment, or uncertainty in the estimated quantities and unit prices for labor, equipment, and materials. Contingency does not cover scope changes nor the Project exclusions noted.

Due to the lack of site-specific geotechnical data, it is difficult to quantitatively estimate the level of accuracy of the design. Therefore, SRK assigned a contingency for each item, based on the level of certainty. For example, for items such as the conventional stacker and grasshopper belts, in which quantities could be reasonably estimated and site-specific vendor quotes could be obtained, a contingency of 30% was assigned. For items such as the Low Permeability Soil Liner in which there was no data to support the quality and quantity assumptions, a contingency of 40% was assigned. The contingency was then weighted by the item costs, and an overall contingency was estimated.

# 9.6 Scope Items

The main cost items included in the HLP cost estimate are as follows:

- Site Preparation;
- Earthworks:

- · Geosynthetics;
- Overliner;
- Piping;
- · Miscellaneous Mechanical Equipment;
- · Closure:
- Construction and Engineering;
- · Contingency; and
- Operational Expenses

#### 9.6.1 Site Preparation

#### Item 110: Mobilization and Demobilization

The Work shall provide for establishment of the site including mobilization of construction equipment, labor, materials, and temporary facilities including the setting up of temporary facilities including field offices, temporary utilities and services, as well as all other related activities and administration and overhead cost relative to site establishment.

Payment shall be lump sum, and was estimated to be 10% of the sum of earthworks, geosynthetics and miscellaneous costs.

Quantities were estimated assuming one mob/demob for one construction period.

#### Item 120: Clear and Grub

The Work shall include clearing and grubbing as required to remove vegetation prior to cutting and filling.

Payment shall be by the hectare.

Quantities were estimated using the ultimate OHLP footprint area, corrected for slope.

#### Item 130: Topsoil Removal and Stockpiling

The Work shall include the excavation, loading, hauling and stockpiling of an average of approximately 300 mm of growth medium prior to cutting and filling within the disturbance limits as shown on the drawings.

Payment shall be by the cubic meter of material.

Quantities were estimated using the ultimate OHLP footprint area, corrected for slope, and multiplied by a topsoil thickness of 300 mm.

#### 9.6.2 Earthworks

#### Item 210: OHLP Over-excavation

The Work will include the excavation, loading, hauling, stockpiling and placement of waste or unsuitable foundation material from within the embankment limits of the OHLP.

Payment shall be by the cubic meter of material.

Quantities were estimated assuming 5% of the compacted fill volume.

#### Item 211: SOF Over-excavation

The Work will include the excavation, loading, hauling, stockpiling and placement of waste or unsuitable foundation material from within the embankment limits of the SOF pad.

Payment shall be by the cubic meter of material.

Quantities were estimated assuming 5% of the compacted fill volume.

#### Item 220: OHLP Compacted Fill (Regrading)

The Work shall include the excavation, loading, hauling, placing, moisture conditioning, oversize segregation, and compacting of Structural Fill within the OHLP area. Any cut shall be considered incidental to the Work. The source of compacted fill was assumed to be excess cut material from within the OHLP limits.

Payment shall be by the cubic meter of material.

Quantities were estimated by comparing the existing ground surface to the final regrade surface.

#### Item 221: SOF Compacted Fill (Regrading)

The Work shall include the excavation, loading, hauling, placing, moisture conditioning, oversize segregation, and compacting of Structural Fill within the SOF area. Any cut shall be considered incidental to the Work. The source of compacted fill was assumed to be excess cut material from within the SOF limits.

Payment shall be by the cubic meter of material.

Quantities were estimated by comparing the existing ground surface to the final regrade surface.

#### Item 230: OHLP Subgrade Preparation

The Work associated with the Subgrade Preparation includes the surface preparation prior to geomembrane deployment within the OHLP area.

Payment shall be by the square meter (neatline).

Quantities were estimated using the OHLP geomembrane area, corrected for slope.

#### Item 231: SOF Subgrade Preparation

The Work associated with the Subgrade Preparation includes the surface preparation prior to geomembrane deployment within the SOF area.

Payment shall be by the square meter (neatline).

Quantities were estimated using the SOF geomembrane area, corrected for slope.

#### Item 240: OHLP Soil Liner

The Work includes the excavation of suitable low permeability onsite material, processing and stockpiling for use as Soil Liner Fill. The Work shall include the loading, hauling, placing, grading, scarifying, moisture conditioning, and compacting as required. The borrow source was assumed to be within 3 km from the site.

Payment shall be by the cubic meter, based on a survey of the geomembrane area multiplied by the neatline thickness.

Quantities were estimated using the OHLP geomembrane area, corrected for slope, and multiplied by a neatline thickness of 300 mm.

#### Item 241: SOF Soil Liner

The Work includes the excavation of suitable low permeability onsite material, processing and stockpiling for use as Soil Liner Fill. The Work shall include the loading, hauling, placing, grading, scarifying, moisture conditioning, and compacting as required. The borrow source was assumed to be within 3 km from the site.

Payment shall be by the cubic meter, based on a survey of the geomembrane area multiplied by the neatline thickness.

Quantities were estimated using the SOF geomembrane area, corrected for slope, and multiplied by a neatline thickness of 300 mm.

#### Item 250: OHLP Single Anchor Trench

The Work shall include the excavation, shaping, and backfilling, including the supply and placement of Anchor Trench Backfill, for a single anchor trench. Anchor trenches shall be backfilled after deployment to prevent the geomembrane from pulling out of the anchor trench.

Payment shall be by the lineal meter (neatline).

Quantities were estimated by measuring the perimeter of the OHLP, and Storm Pond geomembrane area.

#### **Item 251: SOF Single Anchor Trench**

The Work shall include the excavation, shaping, and backfilling, including the supply and placement of Anchor Trench Backfill, for a single anchor trench. Anchor trenches shall be backfilled after deployment to prevent the geomembrane from pulling out of the anchor trench.

Payment shall be by the lineal meter (neatline).

Quantities were estimated by measuring the perimeter of the Spent Ore Facility geomembrane area.

#### **Item 255: HLP Double Anchor Trench**

The Work shall include the excavation, shaping, and backfilling, including the supply and placement of Anchor Trench Backfill, for a double anchor trench configuration. Anchor trenches shall be backfilled after deployment to prevent the geomembrane from pulling out of the anchor trench.

Payment shall be by the lineal meter (neatline).

Quantities were estimated by measuring the perimeter of the Pregnant and Rinse Ponds geomembrane area.

#### Item 260: Blasting

The Work shall include the drilling, loading, blasting and excavating of unrippable rock, with unrippable rock defined as material that cannot be ripped with two passes of a CAT D-9 class dozer with a single shank ripper. Oversized material may be hauled to an area approved by Owner at the Contractor's expense.

Blasting calculations are based on a minimum blasting depth of 1 meter with an overblast of 0.5 meter, within the limits of the OHLP and solution ponds. No blasting was assumed within the limits of the SOF. Payment shall be by the cubic meter.

#### 9.6.3 Geosynthetics

#### Item 310: HLP 2.0mm HDPE Geomembrane

The Work shall include the manufacturing, supply and installation of a 2.0 mm smooth HDPE geomembrane. This Work shall include packaging, shipping, freight, unloading, stockpiling, staging, retrieving geomembrane material from storage area, deployment, seaming, testing, and protecting the geomembrane from weather during all phases of construction. Wastage, overlap and material installed in the anchor trenches shall be considered incidental to the Work.

Payment shall be by the square meter (neatline).

Quantities were estimated using the OHLP geomembrane footprint area, corrected for slope.

#### Item 320: OHLP 1.5mm HDPE Geomembrane

The Work shall include the manufacturing, supply and installation of a 1.5 mm smooth HDPE geomembrane. This Work shall include packaging, shipping, freight, unloading, stockpiling, staging, retrieving geomembrane material from storage area, deployment, seaming, testing, and protecting the geomembrane from weather during all phases of construction. Wastage, overlap and material installed in the anchor trenches shall be considered incidental to the Work.

Payment shall be by the square meter (neatline).

Quantities were estimated using the Storm Pond geomembrane footprint area, corrected for slope.

#### Item 321: SOF 1.5mm HDPE Geomembrane

The Work shall include the manufacturing, supply and installation of a 1.5 mm smooth HDPE geomembrane. This Work shall include packaging, shipping, freight, unloading, stockpiling, staging, retrieving geomembrane material from storage area, deployment, seaming, testing, and protecting the geomembrane from weather during all phases of construction. Wastage, overlap and material installed in the anchor trenches shall be considered incidental to the Work.

Payment shall be by the square meter (neatline).

Quantities were estimated using the SOF geomembrane footprint area, corrected for slope.

#### Item 325: Solution Ponds 1.5mm HDPE Geomembrane (Double) and Drain Net

The Work shall include the manufacturing, supply and installation of both the upper and lower a 1.5 mm smooth HDPE geomembrane and drainage geonet. This Work shall include packaging, shipping, freight, unloading, stockpiling, staging, retrieving geomembrane material from storage area, deployment, seaming, testing, and protecting the geomembrane from weather during all

phases of construction. Wastage, overlap and material installed in the anchor trenches shall be considered incidental to the Work.

Payment shall be by the square meter (neatline).

Quantities were estimated using the geomembrane footprint area for the Pregnant and Rinse Ponds, corrected for slope.

#### 9.6.4 Overliner

#### Item 410: OHLP Overliner

The Work shall include the drilling, blasting, crushing, loading, hauling, placement and grading of the Overliner. The borrow source was assumed to be 10 km from site.

Payment shall be by the cubic meter, based on a survey of the geomembrane area multiplied by the neatline thickness.

Quantities were estimated using the OHLP geomembrane area, corrected for slope, and multiplied by a neatline thickness of 1200 mm. SRK assumed that the 300 mm of replacement Overliner would be replaced every two years.

#### Item 411: SOF Overliner

The Work shall include the drilling, blasting, crushing, loading, hauling, placement and grading of the Overliner. The borrow source was assumed to be 10km from site.Payment shall be by the cubic meter, based on a survey of the geomembrane area multiplied by the neatline thickness.

Quantities were estimated using the SOF geomembrane area, corrected for slope, and multiplied by a neatline thickness of 900 mm.

#### **9.6.5** Piping

#### **Item 510: Drainage Piping**

The Work shall include the manufacturing, supply and installation of all Tertiary, Secondary and Primary piping, including fabricated fittings and mechanical connections. Contractors shall ensure that piping is properly located to intercept flows and not moved or damaged during Overliner placement. Depth requirements for crossing, travelling or working above are measured at top of pipe.

Payment shall be by the square meter (neatline).

Quantities were estimated using the OHLP geomembrane footprint area, corrected for slope.

#### 9.6.6 Miscellaneous Mechanical Equipment

#### Item 710: Fixed Conveyor

The Work shall include the manufacturing, supply and installation of a fixed conveyor system. The fixed conveyors are electrically powered and mounted on foundation blocks resting on a prepared earthen surface. Belt width is 915 mm and carrying capacity is 750 tph. The corridor where this belt is located also contains the power supply for the system using 4 kv cable and plugs. Rail road rails on both side of the fixed conveyer are included and are used to support the rail tripper or the rail hopper.

Payment shall be by the lineal meter, and was estimated to be \$2,900, based on a preliminary vendor estimate.

#### Item 720: Rail Tripper

The Work shall include the manufacturing, supply and installation of a rail tripper. The rail tripper mounts over the fixed conveyor on the installed rail road rails and moves the entire length of the heap leach pad. This machine removes ore from the fixed conveyor and transfers it to the grasshopper belts. It moves along the fixed conveyor beltline when a stacking panel, 60 m wide, is completed. All conveyors are electrically powered and 915 mm wide and capable of carrying 750 tph

Payment shall be lump sum, and was estimated to be \$655,000 each, based on a preliminary vendor estimate.

#### Item 760: Rail Hopper

The Work shall include the manufacturing, supply and installation of a rail hopper. The rail hopper mounts on the installed rail road rails of the fixed conveyor and moves the entire length of the heap leach pad. This hopper receives spent ore from the grasshopper belts and transfers it to the fixed conveyor. It moves along the fixed conveyor beltline when a stacking panel, 60 m wide, is completed. All conveyors are electrically powered and 915 mm wide and capable of carrying 750 tph

Payment shall be lump sum, and was estimated to be \$343,000 each, based on a preliminary vendor estimate.

#### **Item 761: Mobile Conveyor Hopper**

The Work shall include the manufacturing, supply and installation of a mobile conveyor hopper. The hopper receives spent ore from the front end loader and transfers it, using an integral elevating flight chain feeder, to the grasshopper belts. Capacity is 750 tph. The machine is electrically powered.

Payment shall be lump sum, and was estimated to be \$1,300,000 each, based on a preliminary vendor estimate.

#### Item 765: Grasshoppers

The Work shall include the manufacturing, supply and installation of grasshopper conveyor belts. Grasshoppers are 35 m long with 915 mm belt width, electrically powered with 4.1 kv cables and onboard transformers for 480/3. The grasshoppers receive material from the rail tripper and are added as needed to reach the far end of a panel. They have a rigid frame, a feed hopper, a discharge chute and a set of high flotation tires for easy movement. Each grasshopper has a capacity of 750 tph. All grasshopper belts used for stacking and spent ore functions are identical.

Payment shall be lump sum, and was estimated to be \$148,000 each, based on a preliminary vendor estimate.

#### Item 766: Horizontal Feed Conveyor

The work includes the supply and installation of a horizontal feed conveyor. This 27.5 m long belt elevates the ore from the discharge from the last grasshopper and discharges it onto the horizontal conveyor.

Payment shall be lump sum, and was estimated to be \$244,000 each, based on a preliminary vendor estimate.

### Item 767: Horizontal Conveyor

The work includes the supply and installation of a horizontal conveyor. This belt is able to be loaded anywhere along its length from the horizontal feed conveyor and thus allows continuous operation as ore is placed onto the leach pad. The discharge from this conveyor ties to the feed point of the conventional stacker. The conveyor is 915 mm wide and has a capacity of 750 tph and is electrically powered.

Payment shall be lump sum, and was estimated to be \$694,000 each, based on a preliminary vendor estimate.

### Item 770: Conventional Stacker

The Work shall include the manufacturing, supply and installation of a conventional stacker. The stacker is the last belt in the system, receiving ore from the horizontal conveyor. In operation, the conventional stacker slowly places ore onto the OHLP in an arc. The height and speed are hydraulically adjustable and probes are mounted on the end to automatically slew the position of the discharge. The stacker is track mounted for stability and to prevent damage to the pad lining system. The stacker includes a 6 m long stinger belt that is used to fill voids and create a smooth surface. The stacker uses a 915 mm wide conveyor belt and is 36.5 m long with the stinger retracted and has a capacity of 750 tph.

Payment shall be lump sum, and was estimated to be \$774,000 each, based on a preliminary vendor estimate.

### Item 771: Radial Stacker

The Work shall include the manufacturing, supply and installation of a 35 m long radial stacker with a 915 mm wide belt. The stacker has a fixed height and receiving hopper. The radial stacker has hydraulically powered rubber tires that traverse a 90 degree arc to build a stockpile. A concrete path is used where the wheels run. The capacity is 750 tph.

Payment shall be lump sum, and was estimated to be \$465,000 each, based on a preliminary vendor estimate.

### Item 780: 20 Tonne Haul Truck

The Work shall include the manufacturing, supply and delivery of a 20 t haul truck. Twenty-tonne capacity articulated haul trucks were chosen for this analysis. The haul cycle was calculated based upon a 3.5 km round trip and an average speed of 25 kph.

Payment shall be lump sum, and was estimated to be \$375,000 each, based on a preliminary vendor estimate.

### Item 781: CAT 958 Loader

The Work shall include the manufacturing, supply and delivery of a CAT 958 loader (or equivalent). Cycle times were calculated to check the size of rubber tire loader necessary to meet an 11ktpd schedule, using a 6.9 m³ bucket.

Payment shall be lump sum, and was estimated to be \$730,000 each, based on a preliminary vendor estimate.

### Item 782: CAT 12M Grader

The Work shall include the manufacturing, supply and delivery of a CAT 12M motor grader (or equivalent).

Payment shall be lump sum, and was estimated to be \$320,000 each, based on a preliminary vendor estimate.

### Item 783: 20m3 Water Truck

The Work shall include the manufacturing, supply and delivery of 20 m<sup>3</sup> water truck.

Payment shall be lump sum, and was estimated to be \$275,000 each, based on a preliminary vendor estimate.

### Item 784: CAT D7 Dozer

The Work shall include the manufacturing, supply and installation of a CAT D7 dozer (or equivalent).

Payment shall be lump sum, and was estimated to be \$570,000 each, based on a preliminary vendor estimate.

### Item 790: Overland Conveyor

The Work shall include the manufacturing, supply and installation of an overland conveyor system. The overland conveyors are electrically powered and mounted on foundation blocks resting on a prepared earthen surface. Belt width is 915 mm and carrying capacity is 750 tph. The belt corridor also includes the power supply for the system.

Payment shall be by the lineal meter, and was estimated to be \$2,350, based on a preliminary vendor estimate.

### 9.6.7 Closure

### Item 810: Mobilization and Demobilization

The Work shall provide for reclamation of the site including mobilization of construction equipment, labor, materials, and temporary facilities including the setting up of temporary facilities including field offices, temporary utilities and services, as well as all other related activities and administration and overhead cost relative to reclamation.

Payment shall be lump sum, and was estimated to be 10% of the sum of earthworks, geosynthetics and miscellaneous costs based on preliminary reclamation designs.

Quantities were estimated assuming one mob/demob for one construction period.

### **Item 820: Site Reclamation Regrading**

The Work shall include the regrading of the OHLP (after removal of the geomembrane), roadways, conveyor alignments (after removal of the conveyors), and the regarding of the final SOF including

side slopes, roadways, and re-vegetated crest area, as needed to conform to final site reclamation designs.

Payment shall be by the square meter based on the disturbed surface areas within the boundaries of the OHLP and SOF.

### **Item 830: Rock Armour Placement**

The Work shall include the drilling, blasting, crushing, loading, hauling, placement and grading of the rock armour. The borrow source was assumed to be offsite.

Payment shall be by the cubic meter, based on a survey of the SOF slope.

Quantities were estimated using the reclaimed SOF slope area, with one meter strip on the top of the SOF inward from the SOF crest. The rock armour placement is assumed to be  $D_{50}$  = 250 mm material, 500 mm in thickness.

### **Item 840: Topsoil Placement**

The Work includes the loading, hauling, placing, and grading of topsoil on the SOF crest area and reclaimed OHLP footprint area. The topsoil stockpile was assumed to be onsite.

Payment shall be by the cubic meter, based on a survey of the disturbed OHLP and SOF areas multiplied by a thickness of 300 mm.

### **Item 841: Structure Removal Conveyors**

The Work includes the removal of all fixed conveyor systems including dismantlement, breakdown, and removal from the site to an approved reclamation area.

Payment shall be by the lineal meter and is based on an estimated six person crew cost at \$40 per lineal meter.

### Item 842: General Labor Crew

The Work includes the removal of the OHLP geomembrane and piping, and Solution Pond geomembrane. SRK developed the estimate assuming fourteen weeks of time, at a cost \$1200 per day for a six man crew.

Payment was estimated to be lump sum.

### 9.6.8 Construction and Engineering

### Item 910: Engineering

The Work shall provide for the feasibility (permitting) and Issued for Construction (IFC) design and drawings, along with specifications.

Payment shall be lump sum, and was estimated to be 2% of the sum of site preparation, earthworks, geosynthetics and miscellaneous costs.

### Item 920: CM and CQA

The Work shall provide for the construction management and quality assurance required during construction.

Payment shall be lump sum, and was estimated to be 10% of the sum of site preparation, earthworks, geosynthetics and miscellaneous costs.

### Item 930: Owner Costs

The Work shall provide for owner costs required during construction.

Payment shall be lump sum, and was estimated to be 10% of the sum of site preparation, earthworks, geosynthetics and miscellaneous costs.

### 9.6.9 Operational Expenditures

### Item 1000: Operational Expenditures

The operational expenditures shall reflect the cost of placing the fresh ore onto the OHLP, removal and handling of spent ore from the OHLP to the SOF.

Payment is based on a per ton basis based on the summation of the individual operating expenditures.

### 10 Capital and Operating Cost Estimate

Based on the cost model noted in Section 9, SRK developed the Life of Mine (LoM) capital and operating cost estimate summarized in Table 10-1, for an OHLP that considered a conveyor stacking and front end loader reclaim system, sized for an ore height of 4 m, and an SOF-sized to contain the design quantity of 29.2 Mt. The cost estimate includes a contingency (usually 35%) which is typical for a PFS level design. Detailed costs are presented in Appendix E.

Table 10-1: TSF LoM Capital and Operating Cost Estimate

Description	OHLP with Trucks and Loader ('000USD)
Site Preparation	\$4,321
Earthworks	\$3,516
Geosynthetics	\$6,349
Overliner	\$21,665
Piping	\$315
Miscellaneous Mechanical Equipment	\$17,780
Construction and Engineering	\$6,877
Closure	\$3,366
Subtotal Costs	\$64,189
Contingency (35%)	\$22,581
OPEX	\$46,767
Total	\$133,538

### 11 Environmental Considerations

SRK has incorporated numerous Best Management Practises (BMP's) in to the design to minimize solution release to the environment and associated risks. These recommendations are summarized in the following sections.

### 11.1 Liner Leakage Rates

The performance of the composite liner proposed for the OHLP can be approximated by estimating the liner leakage rate or the rate of flow through a geomembrane liner. This is basically an estimate of the volume of water that could be lost through a geomembrane due to defects in the liner that could be introduced in the quality control process during manufacturing, accidental punctures during installation and cover placement, or defects in seams that are not located during construction quality assurance.

Key factors affecting the composite liner performance or liner leakage rate include the following:

- Size of the defect. This is basically the area of an assumed circular shaped defect;
- Hydraulic conductivity of the soil. Hydraulic conductivity values differ greatly depending on the
  material. For example, sand materials have high hydraulic conductivities as water can pass
  through this material fairly easily. However, clay materials tend to have low hydraulic
  conductivities as it is more difficult for water to pass through this material. Therefore, the liner
  leakage rate through a composite liner is significantly less than the liner leakage rate through a
  similar defect in a geomembrane placed on a high-permeability soil like sand or gravel (Giroud
  and Badu-Tweneboah, 1992);
- Solution Head. The head of water is basically the vertical height at which solution is expected to
  "pond" over the geomembrane, i.e. the higher the solution level, the larger the head. A higher
  head basically means that there is greater water pressure at the surface of the liner and this
  would result in more water being pushed through the defect. The water level in a material can
  be lowered by using a free draining granular layer, a piping collection system or both, above the
  liner; and
- Contact quality. The quality of the contact between the geomembrane and low permeability soil is one of the key factors governing the rate of flow through the composite liner, because it governs the radius of the wetted area (Qian, Koerner and Gray 2002). They can be categorized as:
  - Good contact conditions correspond to a geomembrane installed, with as few waves or wrinkles as possible, on top of a low-permeability soil layer that has been adequately compacted and has a smooth surface; and
  - Poor contact conditions correspond to a geomembrane that has been installed with a certain number of waves or wrinkles, and/or placed on a low-permeability soil that has not been well compacted and is not smooth. (Bonaparte, 1989).

The following equations were used in estimating the rate of leakage through a circular hole in the geomembrane component of a composite liner. These are empirical formulas based on analytical studies and model tests (Giroud and Bonaparte, 1989b; Giroud et al., 1989).

For composite liners with 'good' contact conditions, the liner leakage rate can be estimated by the following equation:

$$Q = 0.21 * a^{0.1} * h^{0.9} * k_s^{0.74}$$

For composite liners with 'poor' contact conditions, the liner leakage rate can be estimated by the following equation:

$$Q = 1.15 * a^{0.1} * h^{0.9} * k_s^{0.74}$$

Where:

a is the area of the defect in m<sup>2</sup>

h is the level of water (head) above the liner in m

 $k_s$  is the hydraulic conductivity of the underlaying soil in m/sec

Using the equations referenced above, the following assumptions were then used in estimating liner leakage rates for the Project:

- The solution head acting on the liner was 300 mm;
- A single 1 cm<sup>2</sup> defect per acre (or 2.2 defects per hectare) of composite liner;
- 'Good' contact between the geomembrane and the soil liner; and
- The hydraulic conductivity of the soil underlaying the geomembrane was 1x10<sup>-6</sup> cm/sec.

The results show that having a clay liner with a hydraulic conductivity of  $1x10^{-6}$  cm/sec and a pipe spacing of 5 meter centers will result in an estimated liner leakage rate per cell of approximately 17  $\ell/d$  for an OHLP. Calculations are presented in Appendix F.

### 12 Conclusions and Recommendations

### 12.1 Conclusions

Using the recommendations from the conceptual report as a basis (SRK, 2011), SRK developed a PFS level designs and cost estimates for an OHLP that considered ore placement in 4 m high lifts, with ore placement and spent ore removal using a grasshopper stacking and reclaiming system. Pregnant Leach Solution (PLS) would be collected and report to a Pregnant Pond. Rinse solution would be collected and report to a Rinse Pond, where it would be used as makeup water in the leaching circuit. Spent ore would be conveyed to a SOF for containment and closure, designed to contain a total capacity of 29.2 Mt.

### 12.2 Recommendations

SRK recommends that the following work be performed to advance the OHLP and SOF to the next level of design:

- Detailed Topographic Survey. A topographic survey in the area of the OHLP should be performed to an accuracy level of +/- one meter. The area should be sufficiently large to account for the diversion channels and any borrow areas:
- Field investigation. Field investigations should be performed for the OHLP footprint area, including characterizing the foundation conditions and potential borrow areas. These investigations include the following:
  - Field reconnaissance. A field reconnaissance should be done to confirm the site conditions and any geologic hazards. The geology within and adjacent to the OHLP area should be mapped, including any faulting that may impact the performance of the structure;
  - Geotechnical investigation. A geotechnical program including boreholes and test pits within the OHLP footprint, in addition to borrow areas should be performed. Samples should also be taken for laboratory testing, such as water content, grain size, Atterberg Limits, permeability, shear strength, interface shear strength testing, liner load testing, etc.;
  - Ore characterization. The largest driver in the OHLP selection is the lift height that has been assumed. Laboratory test work should be done to confirm ore design assumptions, such as the densities, water contents and lift heights; and
- Sideslope configuration. Site-specific peak ground accelerations should be established for the site, so that a pseudo-static stability analysis can be done to confirm the sideslopes required from a stability perspective. The sideslopes used in the design should also consider construction and closure requirements;
- Water Balance. SRK requests that the following activities be confirmed:
  - Koza confirm the assumptions made in the water balance calculations, such as draindown period to be considered in the draindown volume calculations or the design storm precipitation event;
  - Koza confirm if a Barren Pond is required;
  - The water balance indicated that if rinsing is done at a rate greater than approximately 1.50 l/hr/m², excess water will be generated that will most likely need to be treated and discharged. Therefore, the pore volumes needed for rinsing the ore should be evaluated and the rinsing rate confirmed;
  - The water balance assumed that all precipitation in the winter months reported through the system immediately, with no accounting for accumulation of snow. This will affect the water balance calculations as snow would accumulate during the winter months and spring snow melt could report during April and May, leading to surges of water through the system; and

- Ramp up conditions, which includes ore loading and leaching rates along with solution chemistry and pond recirculation schedules, can impact makeup water requirements quite significantly during start-up. SRK recommends that the water balance be expanded to include ramp up conditions once more definitive ramp up plans have been developed.
- A site specific water balance should be performed for the OHLP that considers the monthly climatological conditions, as well as the OHLP footprint area;
- Closure Requirements. The next stage of design should develop a closure plan and criteria based on National standards and requirements;
- Environmental Baseline Data. Baseline data collection, such as ground and surface water, should be assembled immediately to establish a database and support permitting requirements;
- Climatological data. Site specific daily precipitation data should be reviewed, and design storm events estimated using statistical methods;
- Overliner. A free draining layer of granular material, with a gradation sized to meet the filter criteria with the spent ore, should be developed. The Overliner source should also be characterized;
- OHLP LDS. SRK assumed that no LDS was required under the OHLP or SOF. This assumption should be confirmed against Turkish regulations and permitting requirements;
- Ore Zone. The depth and volume of overburden (waste) and ore should be confirmed as this could be a potential borrow source, as well as the thickness of oxidized ore;
- Soil Liner thickness. SRK assumed that 300 mm of low permeability Soil Liner would be used in the OHLP, Solution Ponds and SOF as well. During subsequent stages of design, the Soil Liner thickness or permeability requirements for the Solution Ponds and SOF could be reconsidered depending on site specific and regulatory requirements;
- Freeboard. The freeboard assumptions should be confirmed;
- Groundwater. SRK had assumed that the groundwater conditions would not impact the design.
   This should be confirmed:
- Surface water. SRK did not design diversion channels around the perimeter of the OHLP or SOF. Surface water controls should be designed and included in the cost estimate, based on site specific storm data; and
- Monitoring program, including piezometers, slope indicators, survey monuments and groundwater monitoring wells should be developed and incorporated into the cost model.

### 13 References

SRK Consulting, Letter titled "Work Order No. 1", February 2011

Datamine International (September 2009), Mineral Resource Estimate for the Himmetdede Prospect, Turkey, Technical Report

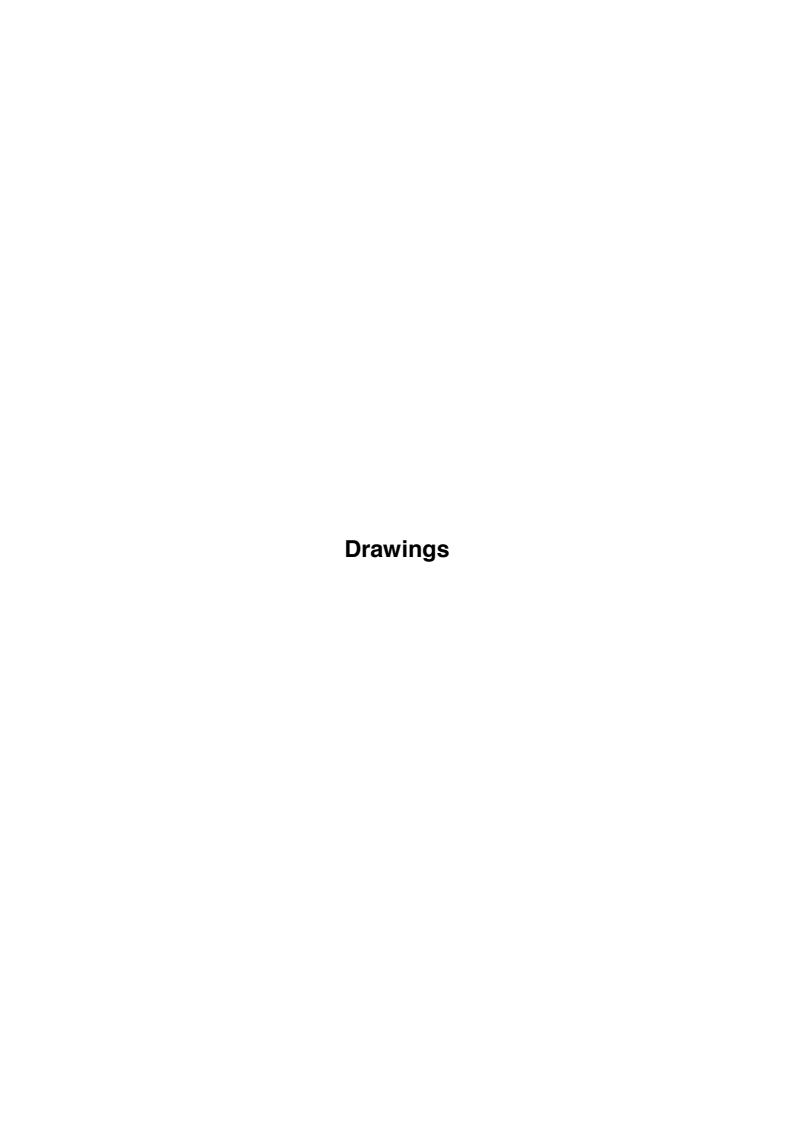
Giroud, J.P. and Bonaparte, R., 1989, "Leakage through Liners Constructed with Geomembranes, Part II: Composite Liners", Geotextiles and Geomembranes, Vol. 8, No.2, pp. 71-111

Giroud, J.P., Khatami, A. and Badu-Tweneboah, K., 1989, "Evaluation of the Rate of Leakage through Composite Liners", Geotextiles and Geomembranes, Vol. 8, No. 4, pp. 337-340.

Giroud, J.P., Badu-Tweneboah, K. and Bonaparte, R., 1992, "Rate of Leakage Through a Composite Liner due to Geomembrane Defects", Geotextiles and Geomembranes, Vol. 11, No. 1, pp. 1-8.

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# HIMMETDEDE PROJECT HEAP LEACH PAD

## LIST OF DRAWINGS

DRAWING	DRAWING TITLE
DRAWING 1	SITE LOCATION MAP AND LIST OF DRAWINGS
DRAWING 2	AREA MAP
DRAWING 3	EXISTING SITE CONDITIONS
DRAWING 4	FACILITY LAYOUT
DRAWING 5	PROJECT AREA GEOLOGY MAP AND FIELD PROGRAM
DRAWING 6	HEAP LEACH PAD PLAN VIEW
DRAWING 7	HEAP LEACH PAD ISOPACH VIEW
DRAWING 8	HEAP LEACH PAD CROSS SECTIONS
DRAWING 9	HEAP LEACH PAD DETAILS
DRAWING 10	SOLUTION PONDS PLAN VIEW
DRAWING 11	SOLUTION PONDS ISOPACH PLAN VIEW
DRAWING 12	SOLUTION PONDS CROSS SECTIONS
DRAWING 13	SOLUTION PONDS DETAILS
DRAWING 14	SPENT ORE FACILITY
DRAWING 15	SPENT ORE FACILITY ISOPACH AND FINAL CONTOURS
DRAWING 16	SPENT ORE FACILITY CROSS SECTIONS





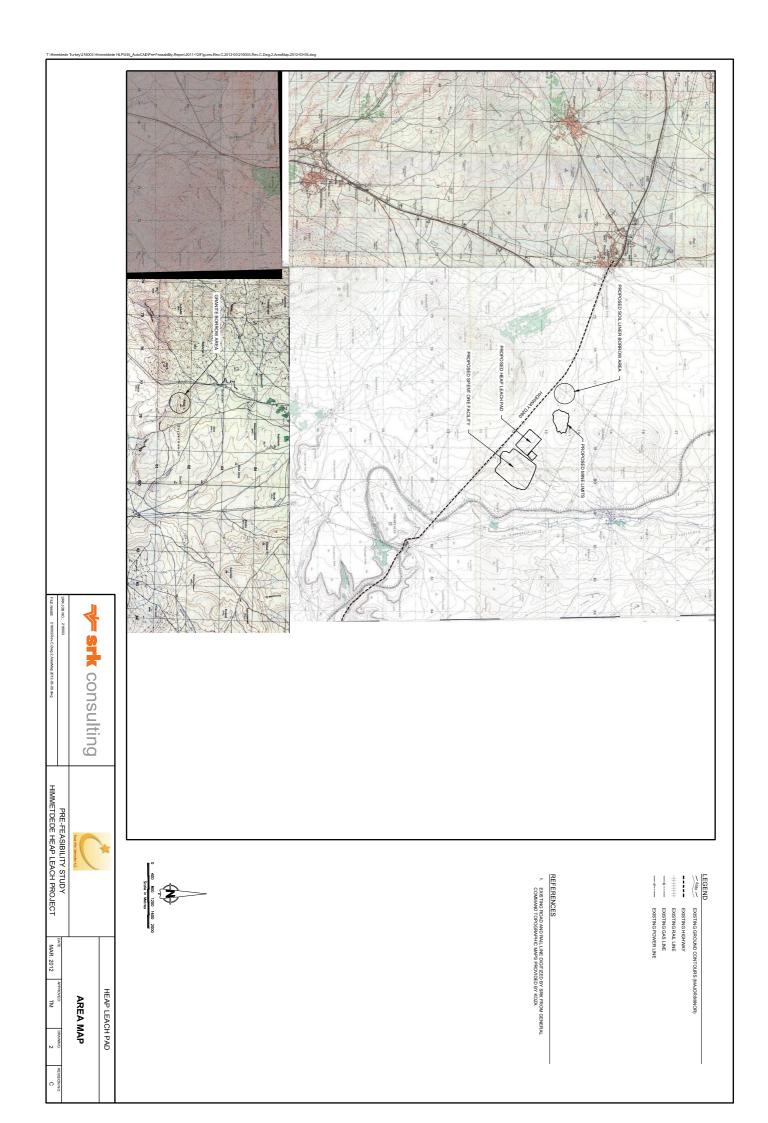


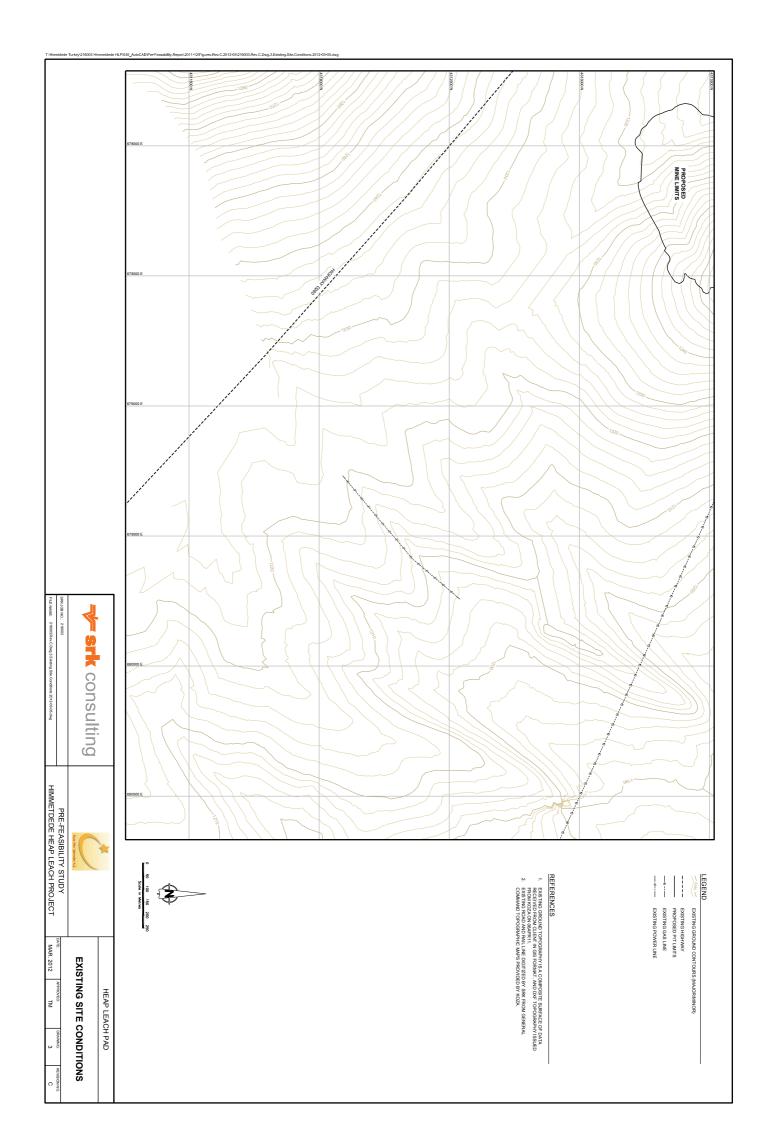


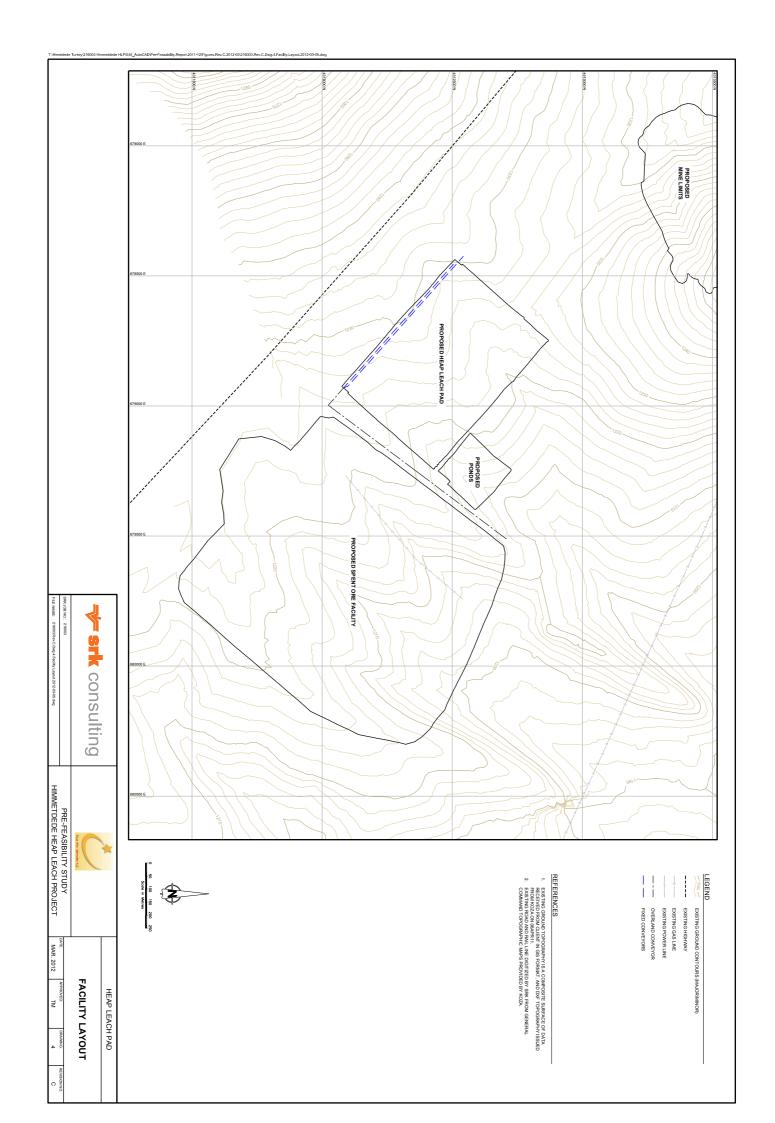
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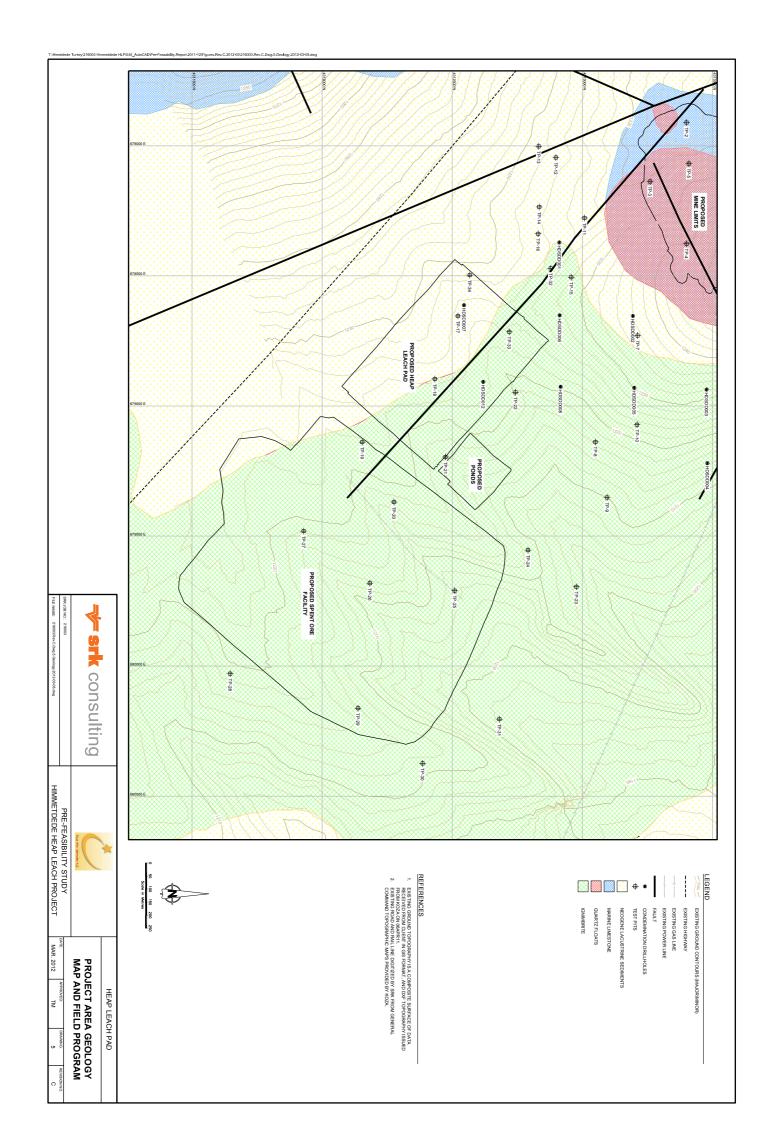
HEAP LEACH PAD

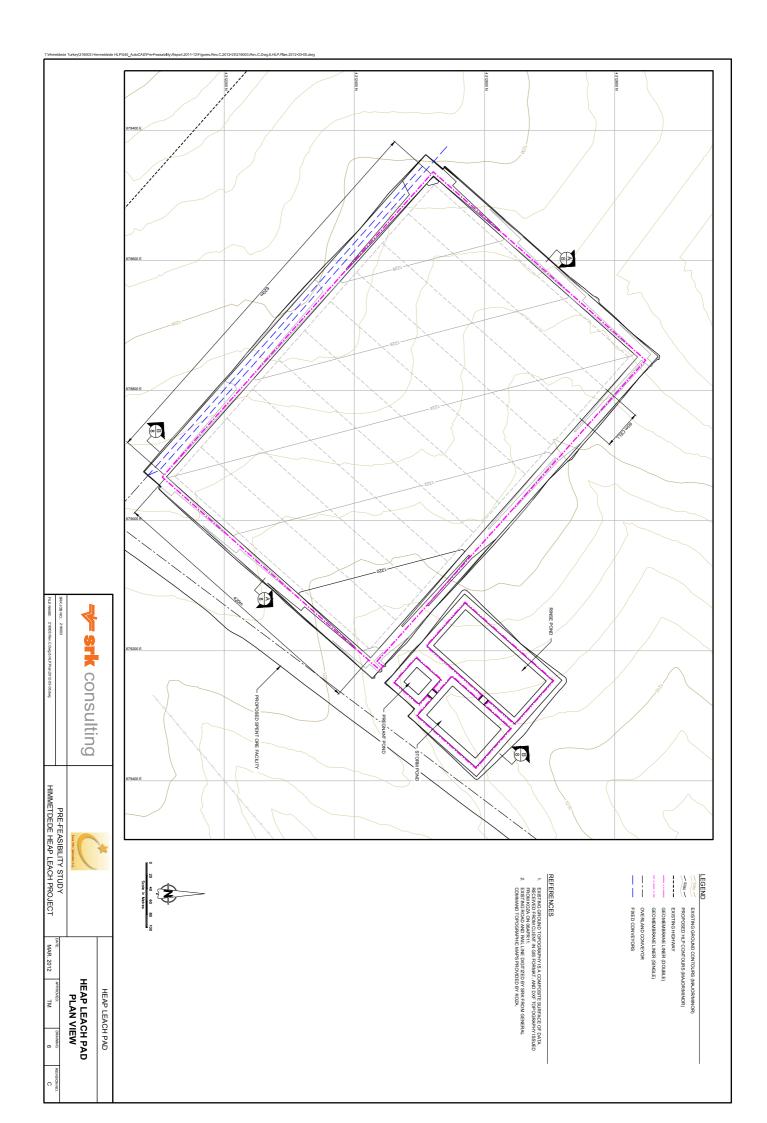
SITE LOCATION MAP AND LIST OF DRAWINGS



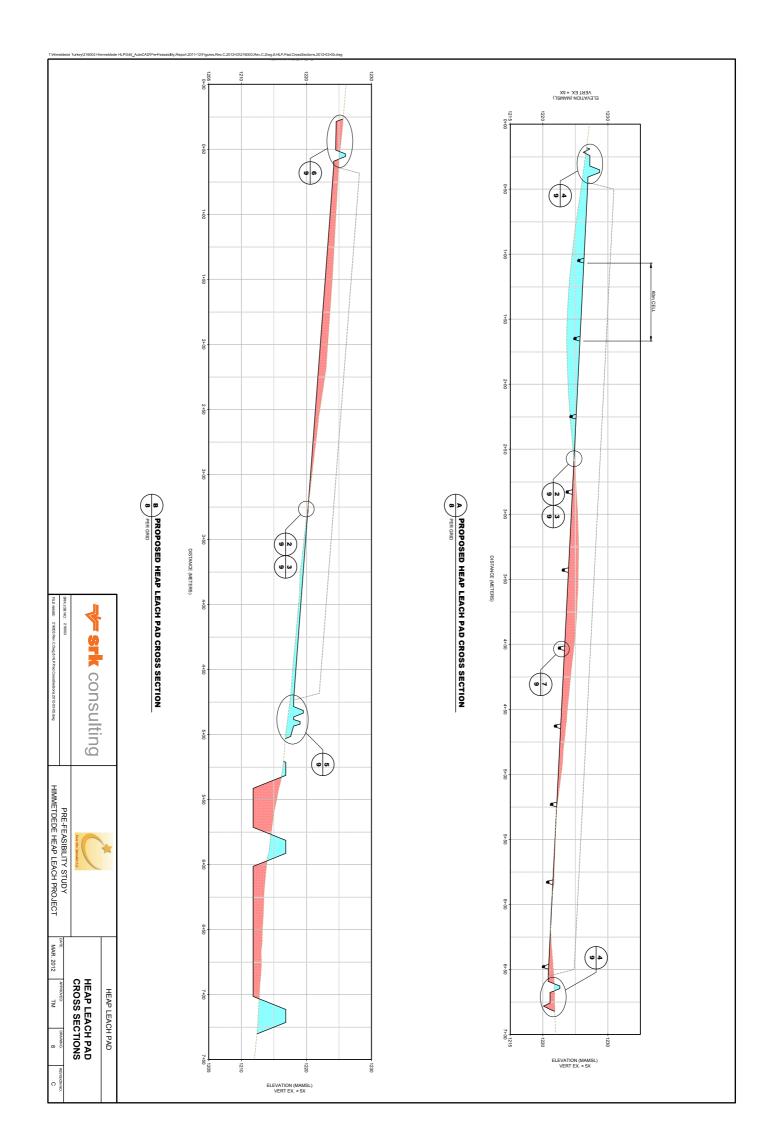


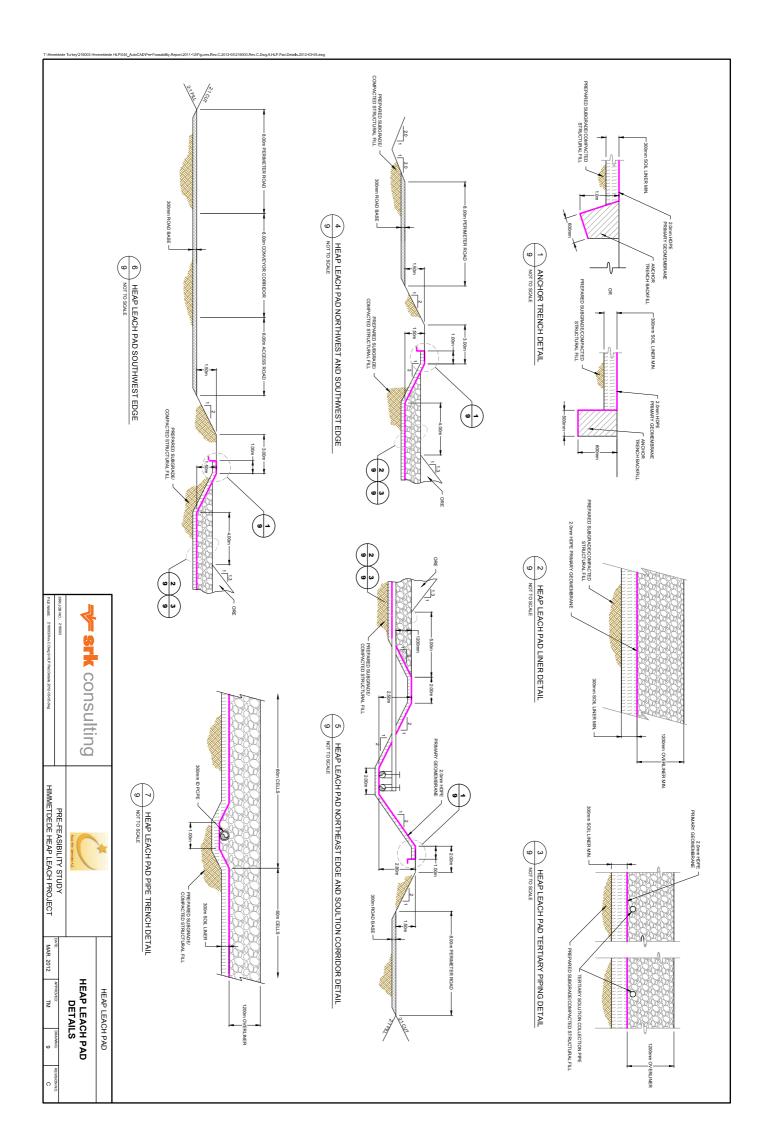


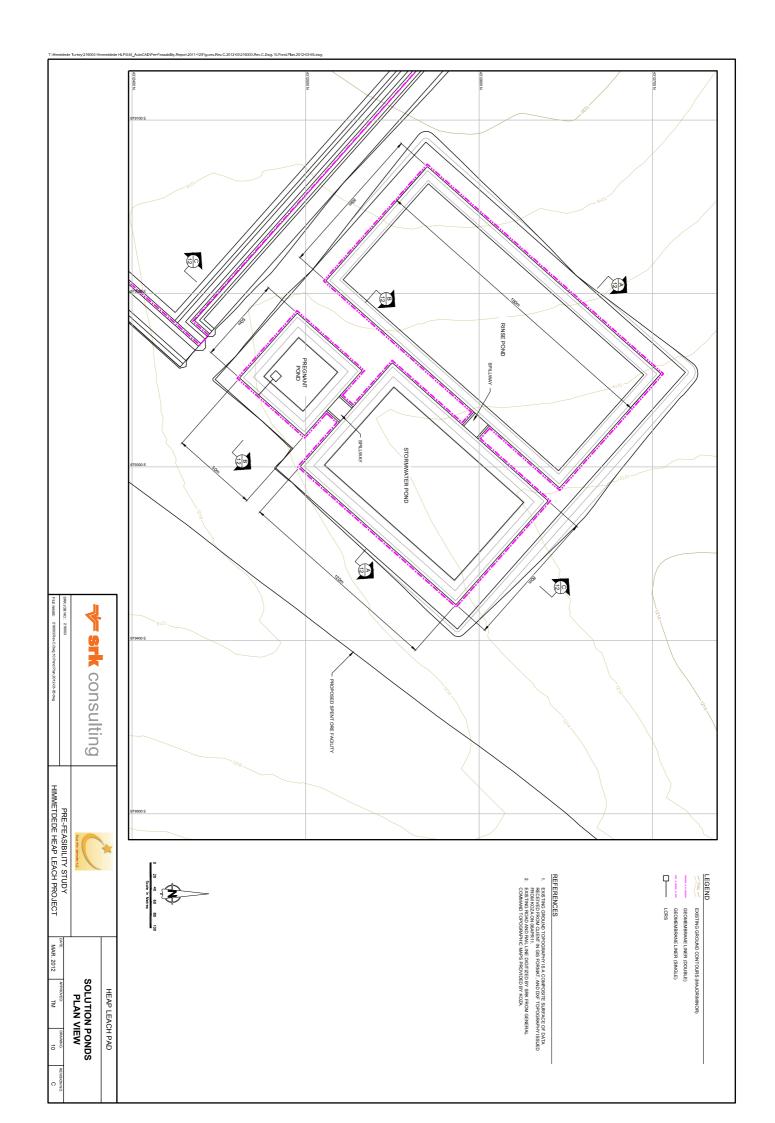


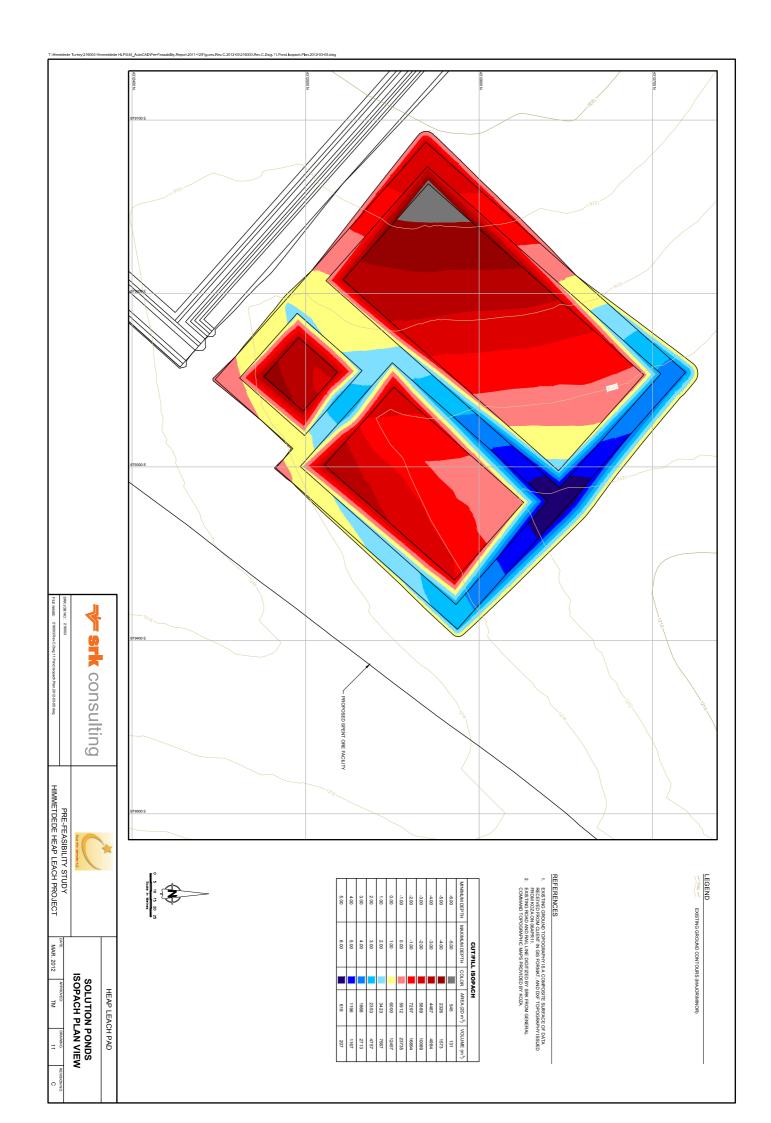


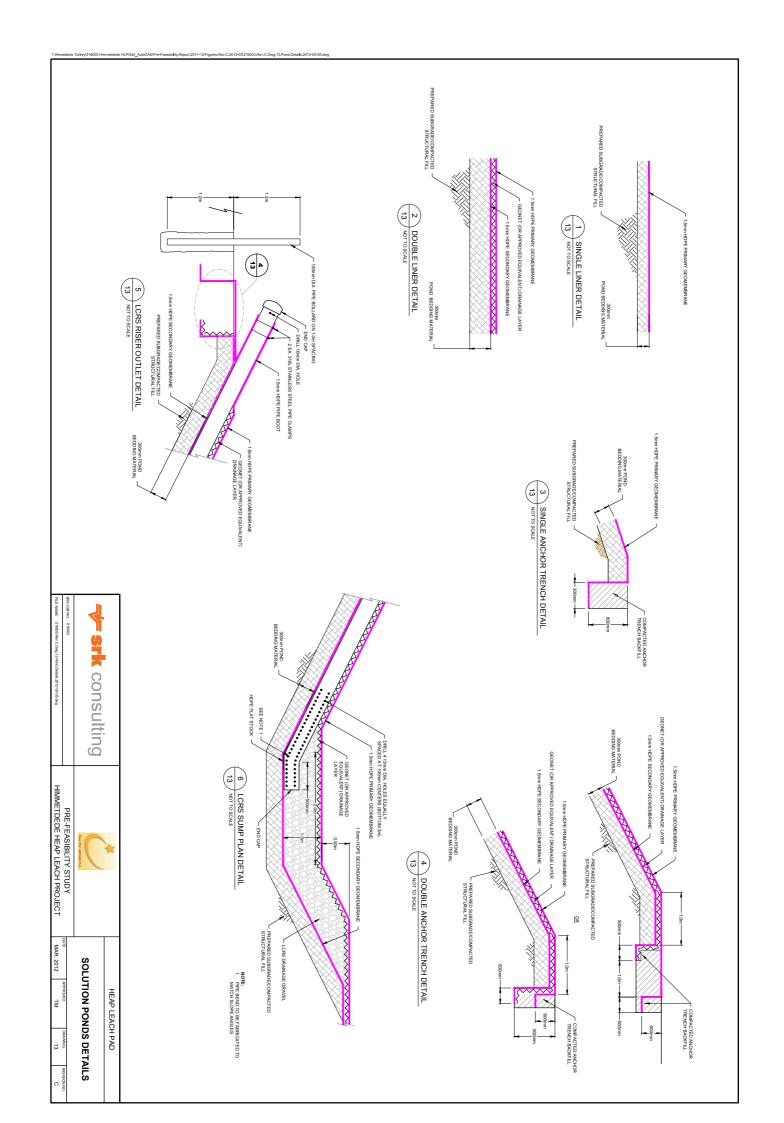


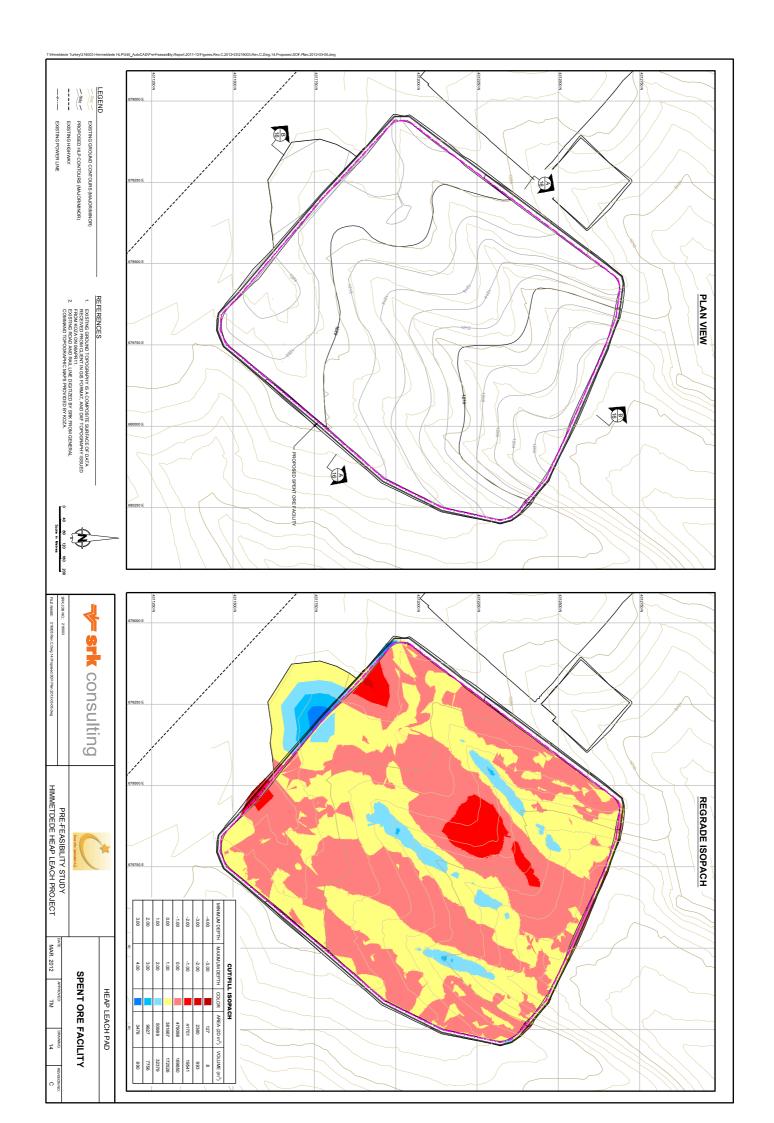


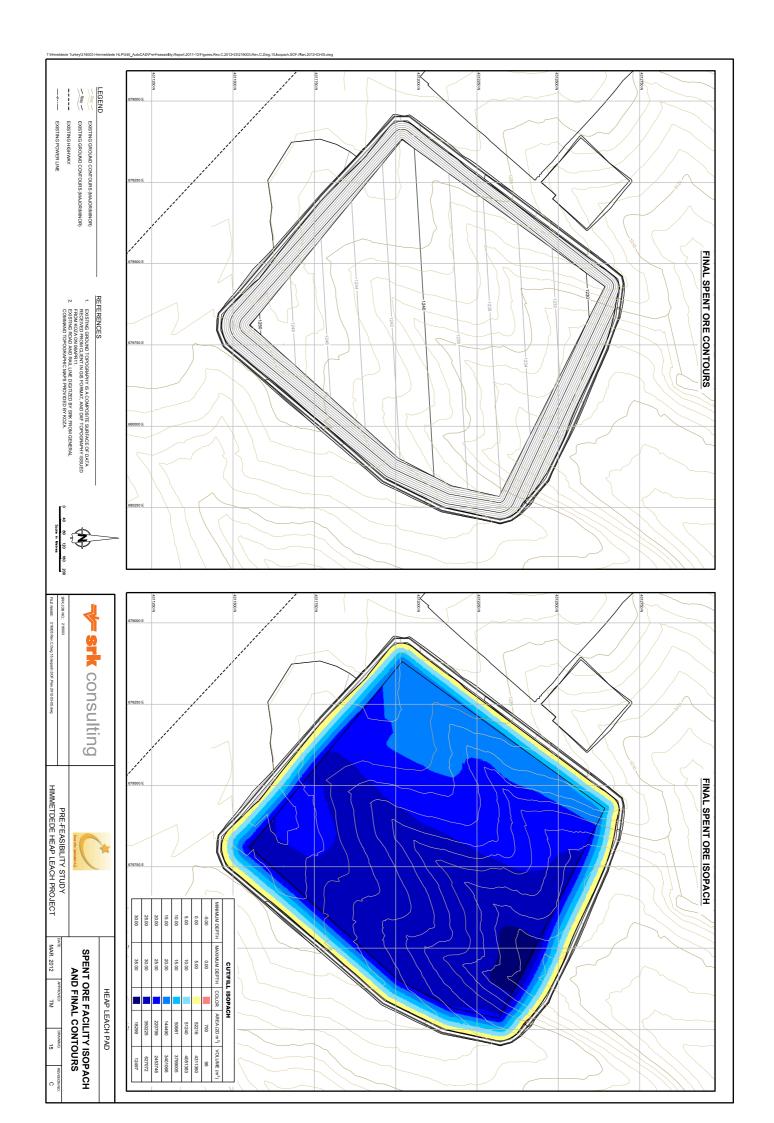


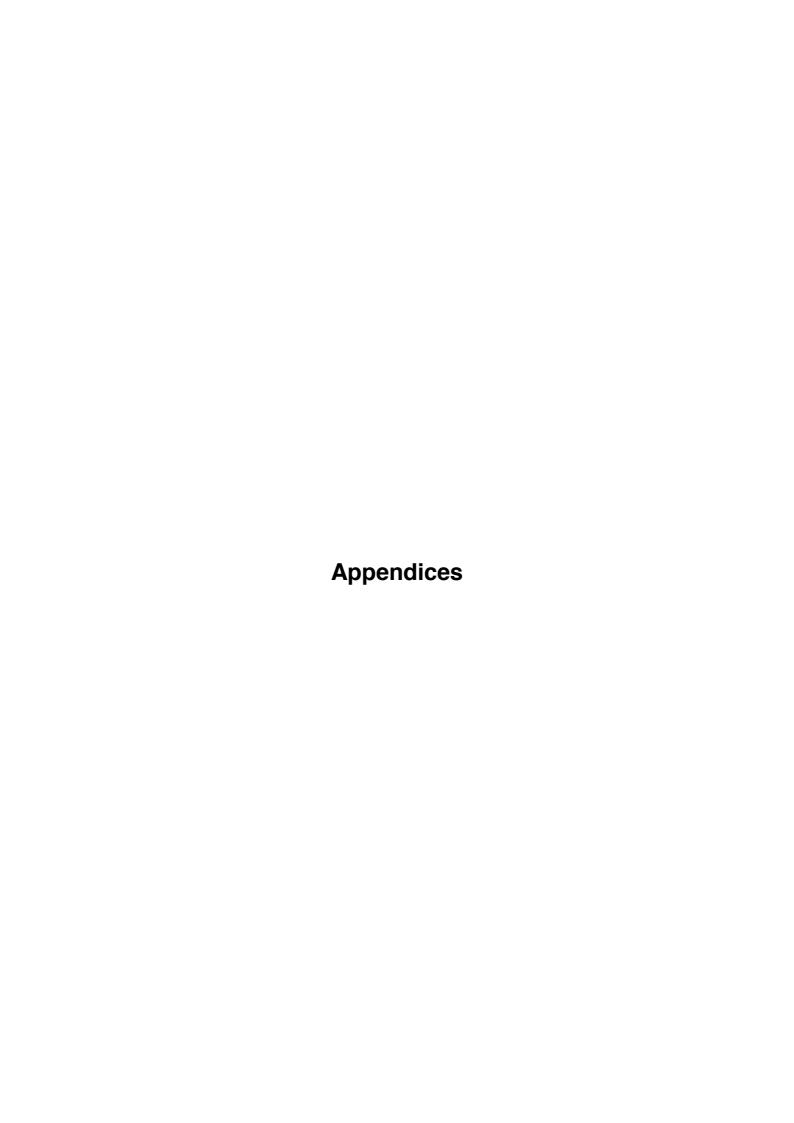












Appendix A:	Design Criteria and Parameters	

# Final Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project

March 12, 2012 SRK Project No. 216003

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							Wet Process Operating Schedule	B2
	18Dec11	Α	M	Α	SRK	750 t/hr	Stacking Design Rate	B1.6
	18Dec11	Α	Μ	Α	SRK	11,000 ktpd	Ore tpd	B1.5
						126 Days	Ramp up Schedule	B1.4
	23May11	A	3	Þ	SRK	2 x 10hr shifts per day	Dry Process Shift Schedule	B1.3
	23May11	A	<b>S</b>	Þ	SRK	20hr/d x 360d/yr	Design utilization	B1.2
	23May11	Α	<b>S</b>	≻	SRK	7,200	Operating shift schedule	B1.1
							Dry Process Operating Schedule	略
				B - Process	B .			
	23May11	Α	Μ	Α	SRK	1.1	Minimum Pseudo-Static FOS	A11
	23May11	Α	M	A	SRK	1.5	Minimum Static FoS (Closure)	A10
	23May11	Þ	≤	Þ	SRK	1.3	Minimum Static FoS (Operations)	A9
	23May11	Α	Μ	Α	SRK		Probabilistic Recurrent Period	A8
	23May11	Α	<b>S</b>	Þ	SRK	2/3 PGA	Design Acceleration	A7
							Peak Ground Acceleration	A6
	23May11	Α	M	Α	SRK	010ct12	Ore Placement Start Date	A5
	23May11	Α	M	A	SRK	Metric	Units	A4
	23May11	Α	Μ	Α	SRK	UTM	Coordinate System	А3
	23May11	Α	M	Α	SRK	35° 5' E	Longitude	A2
	23May11	Α	M	Α	SRK	38° 58' N	Latitude	A1
				A - General	Α-			
Comments	Date	Rev	Level of Confidence	Source / Reference	Design Input	Value Description	Item	Item No.

עבטו	כככ
GN	2
5	5
	3
a	5

MT = Metallurgical Test work

0 = Owner

## Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project Final

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Item No.	Item	Value Description	Design Input	Source / Reference	Level of Confidence	Rev	Date	Comments
B2.1	Operating shift schedule	7,200	SRK	А	≤	>	23May11	
B2.2	Design utilization	20hr/d x 360d/yr	SRK	А	≤	>	23May11	
				Ore				
CI	Ore							
C1.1	Maximum particle size	100% passing 9.5mm	SRK	Α	М	Α	23May11	
C1.2	Work index							
C1.3	Agglomeration	Yes	SRK	Α	Μ	Α	23May11	
C1.4	Amount of Lime Binder	1.75 kg/mt Ore	SRK	С	<b>S</b>	A	23May11	
C1.5	Amount of Cement Binder	1.50 kg/mt Ore	SRK	С	M	Α	23May11	
C2	Ore Grade Design							
СЗ	Ore buffer storage – mined stockpile							
2	Ore buffer storage – crushed stockpile							
C5	Ore Density							
C5.1	Ore bulk density(dry) - crushed ore on conveyor	1600kg/m3	SRK	A	≤	>	23May11	
C5.2	Ore bulk density (dry)- average ore density as stacked on HLP.	1600kg/m3	SRK	Α	≤	>	23May11	
C5.3	Angle of repose	37degrees	SRK	≻	≤	≻	23May11	
C6	Water Content (gravimetric)							
C6.1	As Delivered	3%	SRK	A	3	Þ	5Mar12	
C6.2	Leaching	18%	SRK	А	≤	Þ	5Mar12	
C6.3	Draindown	12%	SRK	A	<b>S</b>	Þ	5Mar12	

Information SRK = SRK Consulting	A = Accumed	C = Calculated	DESIGN GT = Gentechnical Test work	DESIGN INPUT CODES		O = Owner
	A = Assumed P = Published Information	C = Calculated SRK = SRK Consulting	GT = Geotechnical Test work	I = Industry Practice	MT = Metallurgical Test work	O = Owner

## Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project Final

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Item No.	ltem	Value Description	Design Input	Source / Reference	Level of Confidence	Rev	Date	Comments
C7	Shear Strength							
C7.1	Ore Shear Strength							
C7.2	Interface Shear Strength							
			D – He	D – Heap Leach Pad				
D1	Lift height	4m	SRK	Α	Z	A	23May11	
D2	Surface area under leach	120,315 m²	SRK	Α	M	Α	28Dec11	
D3	Number of cells (design) for ultimate configuration	10	SRK	Α	Δ	Α	28Dec11	
D4	Cell Width	60m	SRK	А	Μ	Α	28Dec11	
D5	Leaching Cycle							
D5.1	Stacking	14 days	SRK	А	Μ	Α	23May11	
D5.2	Irrigation	70 days	Koza	MT	Μ	Α	23May11	
D5.3	Draindown	14 days	SRK	A	3	A	23May11	
D5.4	Unloading	14 days	SRK	A	3	Þ	23May11	
D5.5	Contingency	28 days	SRK	A	<b>S</b>	Þ	23May11	
D5.6	Total Cycle	140 days	SRK	A	3	A	23May11	
D6	Percent Recovery (Min.)							
D7	Primary Irrigation rate – I/m²/h (top / slopes)	10	SRK	Α	≤	Α	23May11	
D8	Containment System	Composite liner system comprised of  1200 mm Overliner  2.0mm HDPE Geomembrane  300 Soil Liner	SRK	>	Z	>	23May11	
D9	Pad Type	On-Off	SRK	A	Δ.	Þ	23May11	

mary_12Mar2012_Final_Rev4_BW.doc	03_Koza Himmet_KeyDesignCriteriaSumm	T:\Himetdede Turkey\216003 Himmetdede HLP\071_PFS Design\Report\Report\Report (Rev C Final)\Appendices\A Design Criteria\216003_Koza Himmet_KeyDesignCriteriaSummary_12Mar2012	etdede HLP\071_PFS Design\Report\Re	T:\Himetdede Turkey\216003 Himm
			SRK = SRK Consulting	P = Published Information SRK = SRK Consulting
MT = Metallurgical Test work	<pre>I = Industry Practice</pre>	GT = Geotechnical Test work	C = Calculated	A = Assumed
	DESIGN INPUT CODES	DESIGN		

0 = Owner

# Final Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project

March 12, 2012 SRK Project No. 216003

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	-	:	i	י י י י י י	. ערטוטוא ווא			
				DESIGN INDIT CODES	DESIGN IN			
	23May11	Α	Z	A	SRK	24hours	Draindown (Rinse Pond)	E2.6
	23May11	Α	M	Α	SRK	500mm	Freeboard	E2.5
	1Jan2012	Α	M	Α	SRK	Freeboard Storm Event (27,000 m³)	Storm Pond volume	E2.4
	23May11	A	Z	≯	SRK	Live storage Freeboard Delta Climatological Draindown Minimum intake pump depth (38,000 m³)	Rinse Pond volume	E2.3
	23May11	Α	Μ	×	SRK	Live storage Freeboard Delta Climatological Draindown Minimum intake pump depth (7,513 m³)	Pregnant Pond volume	E2.2
	23May11	Α	Μ	Α	SRK	<ol> <li>Pregnant</li> <li>Rinse</li> <li>Storm</li> </ol>	No. of Ponds	E2.1
							Pond Criteria	E2
	28Dec11	Α	Μ	С	SRK	1200 m³/hr	PLS Flow nominal	E1
				Solution Ponds	E - Sol			
							Maximum HLP Allowable Ground Contact Pressure	D13
	28Dec11	Α	Μ	Α	SRK	Loader / Belts	Pad Off-loading System	D11
	28Dec11	Α	<b>S</b>	Α	SRK	Stacker	Pad On loading System	D10
Comments	Date	Rev	Level of Confidence	Source / Reference	Design Input	Value Description	Item	Item No.

MT = Metallurgical Test work

0 = Owner

# Final Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project

March 12, 2012 SRK Project No. 216003

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						:		
location)						רט ר	Mar	
(about 40kms from HLP	Polylay I	)	=			-5.3	Feb	(1
Kayseri weather station	23Mav11	Þ	3		SRK	-6.9	Jan	3
Climatological data from							Minimum	
							Air temperature (°C)	G1
				Climate Data	G-C			
	23May11	A	<	A	SRK	<ul> <li>Overliner</li> <li>1.5 mm HDPE</li> <li>Geomembrane</li> <li>300 Soil Liner</li> </ul>	Containment	F1
						Composite liner system comprised of		
				F - Spent Ore	Ţ			
	23May11	≻	≤	Þ	SRK	<ul><li>1.5mm HDPE Geomembrane</li><li>300 Soil Liner</li></ul>	Storm Pond Containment	E3
						Single liner system comprised of		
						300 Compacted subgrade		
	,					<ul> <li>1.5mm HDPE</li> <li>Geomembrane</li> </ul>	COLLINE	
	23May11	≻	<b>S</b>	Þ	SRK	Geonet	Pregnant and Rinse Pond	E3
						<ul> <li>1.5mm HDPE</li> <li>Geomembrane</li> </ul>		
						Double liner system comprised of		
	23May11	Þ	<	Α	SRK	2m	Minimum intake pump depth	E2.9
	23May11	Α	M	Α	SRK	100yr, 24 hour	Design Storm	E2.8
	23May11	Α	Z	Α	SRK	6 Hours	Draindown (Pregnant Pond)	E2.7
Comments	Date	Rev	Level of Confidence	Source / Reference	Design Input	Value Description	Item	Item No.

A = Assumed C = Calculated GT = Geotechnical Test work I = Industry Practice MT = Metallurgical Test v
P = Published Information SRK = SRK Consulting
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## Final Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project

March 12, 2012 SRK Project No. 216003

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Ī	G7	G6	G5	G4							ć	3																	Item No.
	From inactive leaching areas	From active leaching	Average Annual Evaporation	24 hr Storm for 100 yr retum period	Yearly Maximum	Dec	Nov	Oct	Sep	Aug	July	June	May	Apr	Mar	Feb	Jan	Maximum	Yearly Minimum	Dec	Nov	Oct	Sep	Aug	July	June	May	Apr	Item
		1594 m³ / cycle		100mm	30.6	5.8	12.2	20.2	26.5	30.6	30.5	26.7	22.1	17.6	11.5	5.7	3.8		-6.9	-4.7	-1.1	3.8	7.4	11.5	12.1	9.7	6.8	3.5	Value Description
		SRK		SRK							CIVIX	SBK																	Design Input
71000		С		Α							C	7																	Source / Reference
		M		М							3	3																	Level of Confidence
		Α		Α							)	>																	Rev
		1Feb12		5Mar12		23May11																	Date						
										location)	(about 40kms from HLP	Kayseri weather station	Climatological data from																Comments

DESIGN INPUT CODES

MT = Metallurgical Test work

0 = Owner

								G12							G11							G10							69	G8	Item No.
	Annual Total	Dec	Nov	Cal		V ::	Aug	July	June	May	Apr	Mar	Feb	Jan	Monthly Average Evaporation	Annual Total	Dec	Nov	Oct	Sep	Aug	July	June	May	Apr	Mar	Feb	Jan	Monthly Average Precipitation	From ponds	Item
	984.2	0	7.7	4.7	68 O	140.4	201.6	216.5	172.0	124.5	58.0	0.6	0	0		397.4	39.4	38.2	33.3	11.4	6.5	13.1	36.0	57.4	56.3	41.1	33.0	31.7			Value Description
DESIGN IN								SRK														SRK									Design Input
DESIGN INDIT CODES																															Source / Reference
								Ζ														≤									Level of Confidence
								≻														Þ									Rev
								23May11														23May11									Date
							location)	(about 40kms from HLP	Climatological data from												location)	(about 40kms from HLP	Climatological data from								Comments

DESIGN INPUT CODES

GT = Geotechnical Test work I = Industry Practice

MT = Metallurgical Test work

0 = Owner

A = Assumed C = Calculated GT = Geotechnical Test work I = Industry Practice MT = Metallurgical Test ν P = Published Information SRK = SRK Consulting Τ.\Himmetdede Turkey\216003 Himmetdede HLP\071\_PFS Design\Report\Rep

# Final Design Criteria and Design Basis Summary Himmetdede Heap Leach Pad Project

March 12, 2012 SRK Project No. 216003

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Item No.	Item	Value Description	Design Input	Source / Reference	Level of Confidence	Rev	Date	Comments
				H - Mining				
포	Operating Schedule							
H1.1	Hours per year	7,200 hr/yr	SRK		Н	Α	23May11	
H1.2	Operating shift schedule	20hr/d x 360d/yr	SRK		I	⊳	23May11	
H1.3	Mining shift schedule	2 x 10hr shifts per day	SRK		Н	Α	23May11	
Н2	Ore production rate							
	Yr-1	0						
	Yr 1	4.0Mtpa / 11ktpd						
	Yr 2	4.0Mtpa / 11ktpd						
	Yr 3	4.0Mtpa / 11ktpd						
H2.1	Yr 4	4.0Mtpa / 11ktpd	0		I	В	08Aug11	
	Yr 5	4.0Mtpa / 11ktpd						
	Yr6	4.0Mtpa / 11ktpd						
	Yr 7	4.0Mtpa / 11ktpd						
	Yr 8	1.2Mtpa / 11ktpd						
H2.2	Reserve	29.2Mt	0		I	≻	23May11	

Appendix B: Phase I Geotechnical Report



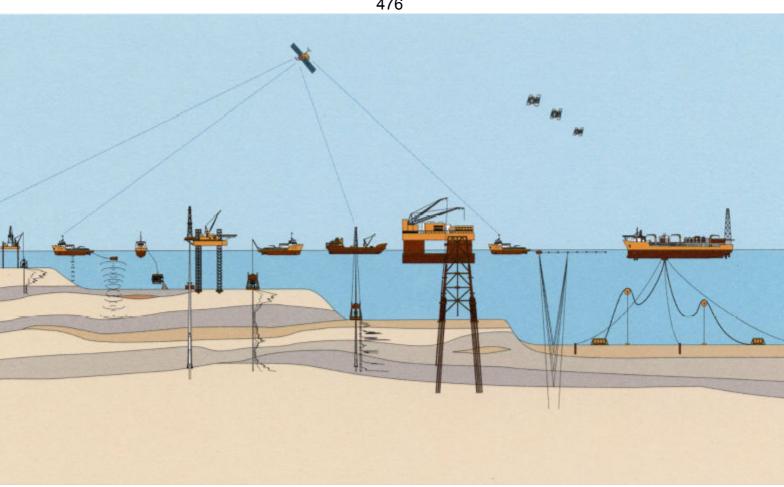
## **PHASE-I**

## **FACTUAL REPORT FOR SUBSURFACE INVESTIGATION FOR KOZA GOLD MINING PROJECT AT** HIMMETDEDE, KAYSERİ

**KOZA MINING AS** 

December 2011

476



## **CONTENTS**

1.0 INTRODUCTION	3
1.1 PURPOSE OF STUDY	3
1.2 SCOPE OF STUDY	3
1.3 GENERAL DESCRIPTION OF PROJECT AREA	3
2.0 SITE INVESTIGATION	5
3.0 GEOLOGY of THE PROJECT SITE	_
3.1. LUTETIAN FORMATION	7
3.2 NEOGENE FORMATIONS	7
4 I ABORATORY WORKS	g

### **APPENDIXES**

APPENDIX-1; TEST PIT LOGS and TEST PIT PICTURES

APPENDIX-2; LABORATORY RESULTS

#### 1.0 INTRODUCTION

This report represents the results of Phase-I work of Geological and Geotechnical subsurface investigation for the Koza Gold Mining AS's Project site at Himmetdede, Kayseri, in Turkey.

#### 1.1 PURPOSE OF STUDY

The purpose of this study (Phase-I) is to determine the surface and subsurface ground conditions for planning the future drilling surveys at the project site and to obtain preliminary data about the physical properties of ground material. After the completion of drilling survey in Phase-II, the engineer will be provided with sufficient information for designing of most suitable and safe foundations for the gold mining operation complex which has been planned to set in the project site.

#### 1.2 SCOPE OF STUDY

.

The scope of study consists of the following items;

- Collecting information about previous studies done on the project site and maps related with the study area.
- Opening 35 numbers of Test Pits in order to determine the type, thickness, sequence, conditions and properties of the ground materials
- Taking representative bulk samples from the Test Pits for the laboratory works
- Carrying out the necessary laboratory tests on the bulk samples from Test Pits

#### 1.3 GENERAL DESCRIPTION OF PROJECT AREA

The understudy project site is inside the boundary of Kayseri-province /Turkey. It is located to the north west of Kayseri Province (Figure-1). It is possible to arrive to the project site through Ankara-Kayseri Highway and the distance from Kayseri to project area is 50 km. Apart from the open pit region the site is relatively flat. At the open pit area where is in the north west of the site, the elevation rises up to 1259 meters from the sea level.

The main occupation of the population is sheep and cattle breeding and agriculture. Himmetdede and its adjoining areas have a continental climate; the temperature between day and night - summer and winter differs much. The nearest meteorological station is in Kayseri where 50 km east from Himmetdede.



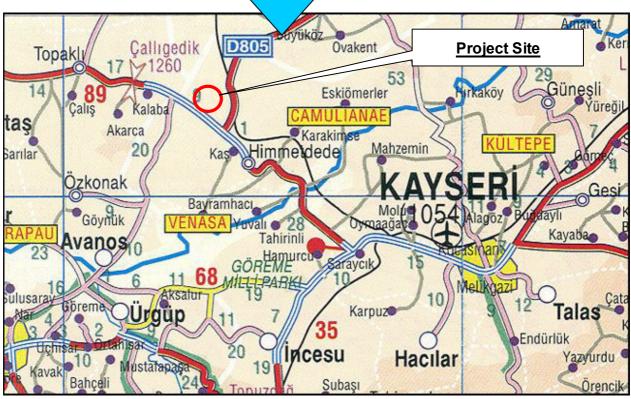


Figure-1: Location of Project Site

According to Meteorological records from Kayseri station;

Average temperature is; 10.4 °C

Average air pressure is; 892.3 hPa

Dominant wind direction is; towards south

Average wind speed is; 1.8 m/s

Average relative humidity is; 64 %

Average precipitation per year is; 397.1 mm

### 2.0 SITE INVESTIGATION

Based on the study scope mentioned earlier, between the dates of 17.10.2011 and 19.10.2011 the site visit was carried out for the visual inspection of the ground conditions and to open 35 numbers of test pits for collecting bulk samples for the laboratory works. After opening the test pits, sub ground conditions were investigated in detail and Test Pit logs were prepared considering the data obtained from the test pits. The list of the test pits are given in Table-1 together with their coordinates.

Test Pit No	North	East
TP-1	4313639	678120
TP-2	4313400	677910
TP-3	4313260	678137
TP-4	4313400	678376
TP-5	4313409	678068
TP-6	4313732	677336
TP-7	4313212	678729
TP-8	4313050	679138
TP-9	4313095	679352
TP-10	4313210	679072
TP-11	4313008	678277
TP-12	4312898	678045
TP-13	4312832	678002
TP-14	4312834	678235
TP-15	4312957	678505
TP-16	4312830	678338
TP-17	4312522	678654
TP-18	4312433	678896
TP-19	4312154	679139
TP-20	4312276	679369
TP-21	4312474	679196
TP-22	4312743	678947
TP-23	4312977	679695
TP-24	4312791	679554
TP-25	4312509	679710
TP-26	4312183	679681
TP-27	4311929	679481
TP-28	4311647	680029
TP-29	4312138	680162
TP-30 (*)	4312386	680374
TP-31	4312681	680204
TP-32	4312878	678471
TP-33	4312719	678715
TP-34	4312567	678497
TP-35	4313450	677373

(\*) Cancelled

Table-1: List of the test pits

Also distribution of the test pit locations at the Project site is shown in Figure-2. Test Pit Logs and pictures of the Test Pits are given in Appendix-1

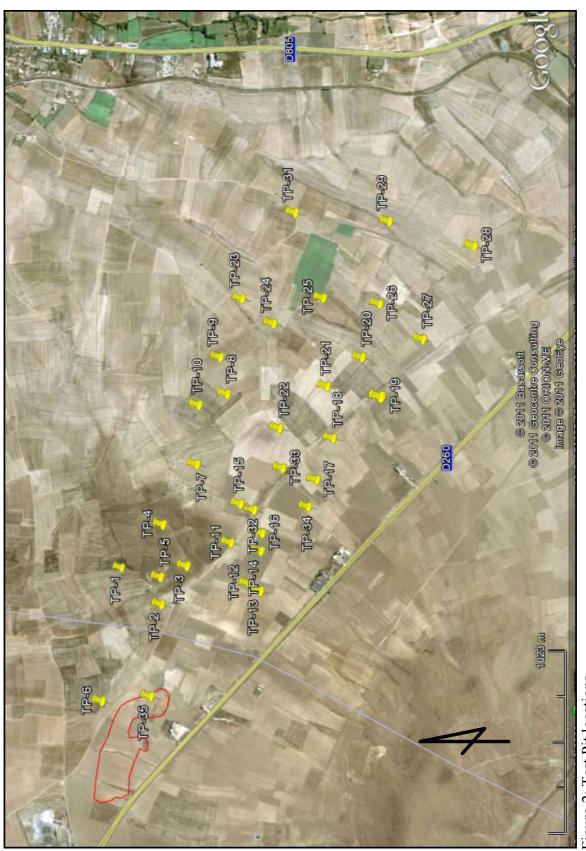


Figure-2; Test Pit locations

#### 3.0 GEOLOGY of THE PROJECT SITE

The geological properties of the Project site is shown in Figure-2, together with layout sketch of mining process complex. In the site Lutetian and Neogene aged geological formations are found. These geological units are demonstrated on the satellite picture taken from Google Earth (Figure-3).

#### 3.1. LUTETIAN FORMATION

The hilly parts of the Himmeddede region consist of marbles and metamorphic rocks. Marbles are coarse- to fine-grained, crystalline and sometimes the presence of breccia zones is observed. The rocks are grey-white, pink or purple. The thickness of these marbles is about 20-30 cm but at places they are thicker and more massive. In some places marble levels are inter bedded with chlorite schistes.

#### **3.2 NEOGENE FORMATIONS**

This formation is composed of: a) Lacustrine limestone, b) tuffs, c) sandy, clayey, calcareous tuffs, Their aged is Upper Miocene.

- a) Lacustrine Limestone; the lacustrine limestone which are generally flaggy (5-10 cm in thickness) or travertine in structure or passing from one to another laterally and vertically. These limestone layers overlie unconformably the marbles and crystalline schist levels.
- b) Tuffs; tuffs are generally 1 m thick and homogeneous in structure. Towards west they contain clay or sandy clayey beds. The color of these tuffs is generally white, pink and rarely black. These tuffs are used as building material all over the area.
- c) Sandy, clayey, calcareous tuffs; these heterogeneous lake deposits generally are seen at the east and northeast of Himmetdede. At the project site, calcareous tuffs were only seen in Test Pits 24, 28 and 29. Due to having limited separation at the project site, these calcareous tuffs were not shown in Figure-3.

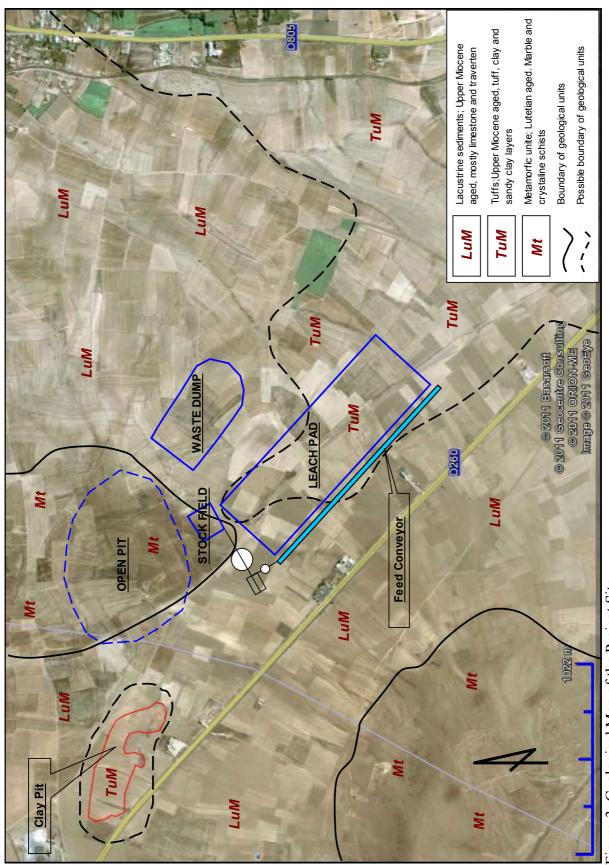


Figure-3; Geological Map of the Project Site

#### 4. LABORATORY WORKS

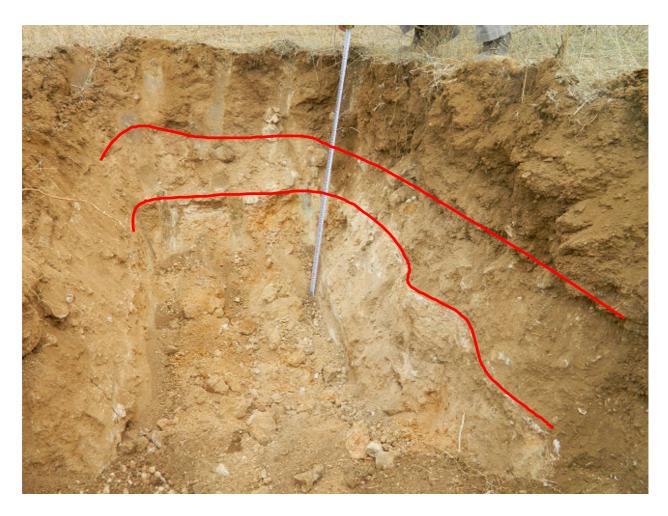
During the test pit works total 30 numbers of bulk samples were collected. On these samples index property tests and soil classification tests were carried out in the Soil Laboratory of EFOL Geotechnical Services in Ankara/Turkey.

Apart from the above mention tests, on the clay samples taken from the test pits 6, 11 and 35 standard compaction and permeability tests were also performed to understand the suitability of the clay materials as being the foundation layer of Leach Pad.

Summary sheets of the laboratory results are given in Appendix-2

APPENDIX-1
TEST PIT LOGS
and
TEST PIT PICTURES

	Uzman Ekip	/ Expert Tea	ım		İş	veren / Cli	ent		Proje /Projec	et .
FUGRO SIAL	FUGRO SİAL MÜŞAVİRLİK V				Ko	oza Madeno	cilik			
	Lit	. yıı.	ARAS	TIRMA (	CUKU	RULOGI	J / TEST PIT LOG			
Çukur No / Test Pi	t No		Airry	TP-1	y O I TO	10 2000	Sayfa No / Page No			1
Müteahhit / Contra							YAS Derinliği / Ground	d water level	(m)	
Çukur Yeri / Pit Lo	cation						Çukur Derinliği / Dept		(m)	1,20
Tarih / Date		17.10.2011					Kot / Elevation	0. 1.10 . 001.1 11	(m)	
Kazı Metodu / Exca	avation Method	by Excavato	or				Koordinatlar / Coordin	actor	N	4.313.639
Makine / Machine		Hidromek 22	20LC				Roordinatial / Coordin	iales	Е	678.120
Derinlik / Depth Numune No / Sample No Numune Derinliği / Depth of Sample		e Deney / u Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m) (m)	Cinsi / Type	Sonuç / Result (kg/cm²)	V- V-							
0,30 m		\rightarrow \right	ر که که که که که که که که که که که که که	TS	Т	op Soil				
0,45 m		33	3333	SM		ght brown gra	vely silty sand			
- S-1	D	***		SIVI	Y	ellowish browr	colored highly weathered ro	ck, soft rock, easy to dig	g by excavator	
1,00 m		88			Y	ellowish brown	colored slightly weathered re	ock, hard to excavate by	y machine	
- 2 - 2 - 3 3 4 5 5 										
Açıklama / Explana		T			7FMİ		im / Plan View:  D	Azimut / Azi A C (m) PARAMETERS	В	(m)
KISALTMALAR /	ABBREVIATION	INCE DA	ANELİ / F	INE GRA			I / COARSE GRAINS		TISITE / PLA	STICITY
D = Torba Numune / CP= Cep Penetrome		N: 3-	- 4 Yum	ımuşak/Very ıuşak / Soft Katı / Mediu		N : 0 - 4 N :5 - 10 N :11 - 30	Gevşek / Loose		Çok Az / Trace of Az / Low Plastic Orta / Medium P	
Pocket Penetrometer  Mühendis	•	N: 9-	- 15 Katı	/ Stiff Katı / Very S		N :31 - 50 N :> 50	Sıkı / Dense		Yüksek / High P Cok Yüksek / Ve	lastic



Test Pit-1

	=	1	Uzm	an Ekip	Expert 1	eam		İş	veren / Cli	ent		Proje /Proje	ct
UG		SIAL		VİRLİK VI	YERBİLİM E MÜHEN ŞTİ.			Ko	oza Maden	cilik			
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG			
uku	r No / T	Γest Pit	No				TP-2			Sayfa No / Page No			1
ütea	ahhit /	Contrac	ctor							YAS Derinliği / Grour	nd water level	(m)	
uku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep		(m)	1,60
arih	/ Date				17.10.20	11				Kot / Elevation		(m)	
azı l	Metodu	ı / Exca	vation N	/lethod	by Excav	ator						N	4.313.400
akir	ne / Ma	chine			Hidromel	< 220LC				Koordinatlar / Coordi	nates	Е	677.910
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / i Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil	
m)		(m)		Туре	Result (kg/cm <sup>2</sup> )								
							TS	To	op Soil				
50 n	n						10						
	S-1		D			- × - x - x - x	ML	Y	ellowish browi	n colored clayey silty fine san	d or low plastic silt, slig	htly wet, medium	dense, Residual so
,40 n	n —					. X		Y	ellowish browi	n colored slightly weatherede	rock, hard to excavate	by machine	
,60 n	n —												
3													
cıkla	oklama / Explanation:								Üst Görün	üm / Plan View:	Azimut / Az	zimuth:	
											A		T
çıklama / Explanation:										D	C (m)	В	(m
ZIQ.	Δ1 TN#4	IAP /	VBBDL/	/IATION				ZEMİ	N PARAN	METRELERİ / SOIL		S	
\IS/	AL I IVIA	ALAK / /	HBBKE/	MATION		DANELİ / I		AINS	IRI DANE	Lİ / COARSE GRAINS	PLA	STİSİTE / PLA	
P= C	Cep Per	etromet	Bulk Sam		N : N : N :	3 - 4 Yur 5 - 8 Orta	'umuşak/Ver nuşak / Soft a Katı / Medi	ium	N : 0 - 4 N :5 - 10 N :11 - 30	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace Az / Low Plastic Orta / Medium F	Plastic
CKE				r	1								
е		rometer hendis /	Enginee	r	N: N:		ı / Stiff k Katı / Very		N :31 - 50 N :> 50	Sıkı / Dense Çok Sıkı / Very Dense	PI :20 - 40% PI :>40%	Yüksek / High F Çok Yüksek / V	Plastic ery High Plastic



Test Pit-2

		-1	Hzm	an Ekin	/ Expert 1	eam		lev	veren / Cli	ent	1	Proje /Proje	ct		
-Fug	RO (	E IAI	1	-	YERBİLİM			ışı	veren / On	ent		r roje /r roje	o.		
•		SIAL		VİRLİK VI	E MÜHEN . ŞTİ.			Ko	za Maden	cilik					
						ARAŞ	TIRMA	ÇUKUF	RU LOGI	J / TEST PIT LOG					
Çuku	r No /	Test Pit	: No				TP-3			Sayfa No / Page No			1		
Mütea	ahhit /	Contra	ctor							YAS Derinliği / Grour	nd water level	(m)			
Çuku	r Yeri /	Pit Loc	cation							Çukur Derinliği / Dep		(m)	2,40		
Tarih	/ Date	:			17.10.20	11				Kot / Elevation		(m)			
Kazı I	Metodi	u / Exca	vation N	/lethod	by Excav	ator						N	4.313.260		
Makir	ne / Ma	achine			Hidromel	< 220LC				Koordinatlar / Coordi	nates	Е	678.137		
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil			
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)										
0.50					,	*** ***	TS		Top Soil						
0,50 n	S-1		D				ML		Darkish brow	n colored, clay or silt, dry to	slightly wet, medium so	ft to soft, residual	soil		
1,50 n	n —														
2	S-2		D				ML		Yellowish bro	wn colored, highly weathered	d rock, partly oxidized,	soft rock			
2,40 n	n —														
3															
4															
5															
•															
5															
								Üst Görünüm / Plan View: Azimut / Azimuth:							
								ľ	ost oorall	a/ Figit view.			<del></del>		
6											Α				
										D		В	(m		
											С		,		
													-		
										· 	(m)	'			
KIS	ALTMA	ALAR /	ABBRE\	/IATION	1					METRELERÍ / SOIL			0710:7		
						DANELI /				LÍ / COARSE GRAINS		STISITE / PLA			
) = To	orba Nu	ımune /	Bulk San	nple	N: N:		'umuşak/Ver muşak / Soft	-	N : 0 - 4 N :5 - 10	Çok Gevşek/Very Loose Gevşek / Loose	PI : 1 - 5% PI :5 - 10%	Çok Az / Trace Az / Low Plastic			
			tre Ölçüm	nü /	N:		a Katı / Medi		N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium I			
'ocke		trometer			N:		tı / Stiff		N :31 - 50	Sıkı / Dense	PI :20 - 40%	Yüksek / High F	Plastic		
	Mü	ihendis /	Enginee	r	N :		k Katı / Very	Stiff	N :> 50	Çok Sıkı / Very Dense	PI :>40%	Çok Yüksek / V	ery High Plastic		



Test Pit-3

	<u> </u>	<del>. T                                   </del>	Uzm	an Ekip /	Expert 7	ream		İş	veren / Cli	ient		Proje /Project	
-Fug		SIAL	FUGI	RO SIAL Y VİRLİK VE	YERBİLİM	1LERİ			oza Maden				
			<u> </u>			ARAS	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
Çukur	No /	Test Pit	No				TP-4			Sayfa No / Page No			1
Mütea	hhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)	
Çukur	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	3,00
Tarih	/ Date				17.10.20	11				Kot / Elevation		(m)	
Kazı I	Metodu	J / Exca	vation M	/lethod	by Excav	rator				Koordinatlar / Coordir	nates	N	4.313.400
Makin	e / Ma	achine			Hidrome	k 220LC				Roordinatial 7 Coordin	iates	E	678.376
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
-						1 V V V	TS		Top Soil				
0,40 m	. —					بلہ بلہ بل <i>ہ</i>			White colored	d carbonate layer, hard crust	(Travertine)		
0,60 m	S-1		D				SM		Yellowish bro	own colored, highly weathered	d rock, soft rock, easy to	excavate by machin	ne
- 2 2 3,00 m													
- 4 5 									To.				
- 6 Açıklama / Explanation:									Üst Görün	üm / Plan View:	Azimut / Azi	B	(m)
NIC	\		<b>ADDDC</b>	/IATION				ZEMİ	N PARAN	METRELERİ / SOIL		3	
NISA	AL I IVIA	ALAK / /	ABBKEV	/IATION		DANELİ /		AINS	IRI DANEI	LÍ / COARSE GRAINS	PLAS	TISITE / PLAST	
CP= C	ep Per	netromet	Bulk Sam tre Ölçüm		N : N : N :	3 - 4 Yur	/umuşak/Ver muşak / Soft a Katı / Medi		N : 0 - 4 N :5 - 10 N :11 - 30	Gevşek / Loose	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace of P Az / Low Plastic Orta / Medium Plast	
Pocke		trometer	Engineer	r	N:		tı / Stiff	Ctiff	N :31 - 50	Siki / Dense	PI :20 - 40%	Yüksek / High Plast	



Test Pit-4

		1	Uzm	an Ekip	Expert 7	ream .		İş	veren / Cli	ent		Proje /Project		
Tug		SIAL	FUG	RO SİAL` VİRLİK VE	YERBİLİM	/LERİ			oza Maden			, ,		
						ARA	STIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG				
Çukuı	No / <sup>-</sup>	Test Pit	No				TP-5			Sayfa No / Page No			1	
_		Contrac								YAS Derinliği / Groun	d water level	(m)		_
Çukuı	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	1,55	_
Tarih	/ Date				17.10.20	11				Kot / Elevation		(m)		
Kazı I	Metodu	u / Exca	avation M	/lethod	by Excav	ator				Koordinatlar / Coordir	nates	N	4.313.409	
Makin	ne / Ma	achine			Hidrome	k 220LC				Roordinatial / Coordin	iates	E	678.068	
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	\L-\L-\	,							
-						, v4, v4, v4, v4,	TS							
- - - 1 -	S-1						SM							
- 2 - -														
- 3 - - -														
-														
- - 5 -														
- - - 6														
-6 Açıklama / Explanation:									Üst Görün	üm / Plan View:	Azimut / Azi	imuth:	(m)	
1/10	A1 T		ADDDE:	UATION				ZEMİ	N PARAN	METRELERİ / SOIL		3		
KISA	λL I MA	ALAR / /	ABBRE\	/IATION	İNCE	DANELİ /	FINE GRA		IRI DANE	LÍ / COARSE GRAINS		TISITE / PLAS	TICITY	
CP= C	Cep Per	netromet	Bulk Sam tre Ölçüm		N : N : N :	3 - 4 Yu	Yumuşak/Ver ımuşak / Soft ta Katı / Medi		N : 0 - 4 N :5 - 10 N :11 - 30	Gevşek / Loose	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace of Az / Low Plastic Orta / Medium Plas		
	t Penet	trometer			N:		atı / Stiff	Ctiff	N :31 - 50		PI :20 - 40%	Yüksek / High Plas		



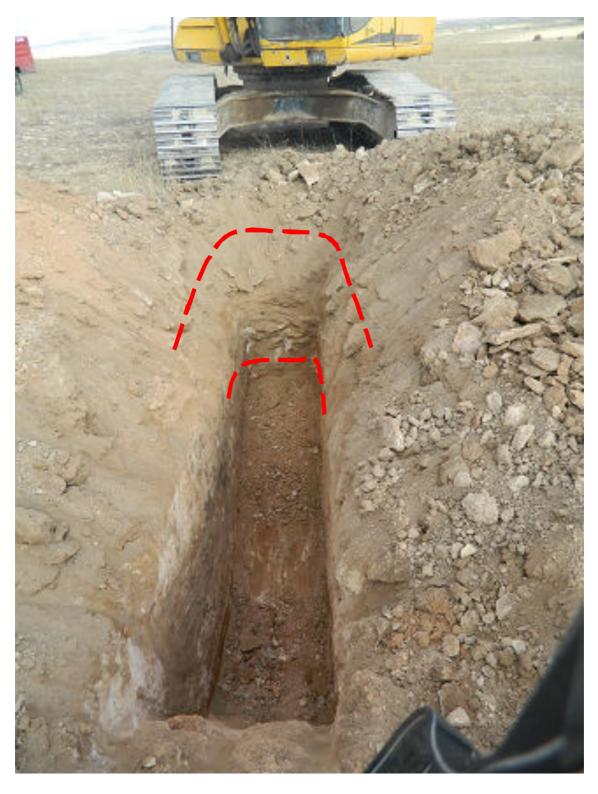
Test Pit-5

Uzman Ekip					-			İş	şveren / Cli	ient	Proje /Project					
		SIAL		VİRLİK VE	YERBİLİM E MÜHENI . ŞTİ.			К	oza Maden	cilik						
						ARAŞ	TIRMA	ÇUKU	RU LOG	U / TEST PIT LOG						
ukur	No / ٦	Γest Pit	No				TP-6			Sayfa No / Page No			1			
Müteahhit / Contractor										YAS Derinliği / Grour	nd water level	ter level (m)				
Cukur Yeri / Pit Location										Çukur Derinliği / Dep	th of the Test Pit					
arih	/ Date				19.10.20	11				Kot / Elevation		(m)				
azı N	Metodu	ı / Exca	vation N	1ethod	by Excav	ator				Koordinatlar / Coordi	N 4.313.732					
akin	ie / Ma	chine			Hidromek	220LC				Troordination 7 Goordi	nated	Е	677.336			
					Zemin Profili / Soil Class Soil Class				Zemin Tanımlam	ası / Description	of the Soil					
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)											
1						1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	TS									
3	S-1		D				МН									
1																
5																
açıkla		Explana						7FMİ		üm / Plan View:  D  METRELERÎ / SOIL	Azimut / Az  A  C  (m)  PARAMETER:	В	(I			
KISA	AL I'MA	ALAR / /	ARRE/	/IATION	İNCE	DANELİ / I	INE GRA			LÍ / COARSE GRAINS		STISITE / PLA	STICITY			
D = Torba Numune / Bulk Sample  CP= Cep Penetrometre Ölçümü / Pocket Penetrometer  Mühendis / Engineer					N: N: N: N:	0 - 2	rumuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff k Katı / Very	y Soft ium	N:0-4 N:5-10 N:11-30 N:31-50 N:>50	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PI: 1 - 5% PI:5 - 10% PI:10 - 20% PI:20 - 40% PI:>40%	Çok Az / Trace Az / Low Plastic Orta / Medium F Yüksek / High F Çok Yüksek / V	of Plasticity Plastic			



Test Pit-6

		1	Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ient		Proje /Project		
Tue		SIAL	FUG	RO SIAL Y VIRLİK VE	YERBİLİM	ILERİ			oza Maden		,,			
			<u> </u>		· · · ·	ARAS	STIRMA	СИКИ	RU LOGI	U / TEST PIT LOG				_
Cukuı	r No / <sup>-</sup>	Test Pit	No				TP-7	3		Sayfa No / Page No			1	_
_		Contrac								YAS Derinliği / Groun	d water level	(m)		_
Çukuı	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	3,00	_
Tarih	/ Date				19.10.20	11				Kot / Elevation		(m)		_
Kazı I	Metodu	u / Exca	avation M	/lethod	by Excav	ator				Koordinatlar / Coordin	notoo	N	4.313.212	
Makin	ne / Ma	achine			Hidrome	( 220LC				Roordinatial / Coordin	iales	Е	678.729	
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class		Zemin Tanımlaması / Description of the Soil					
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
-						بلر بلر بلر	TS							
-														
- 1 - - 2 - - -	S-1		D				SM							
- 4 - - - 5 - - -														
Açıklama / Explanation:										üm / Plan View:  D  HETRELERÎ / SOIL	Azimut / Azi A C (m) PARAMETERS	В	(m)	
KISA	ALTMA	ALAR / /	4BBRE\	/IATION	İNCE	DANELİ /	FINE GRA			LÍ / COARSE GRAINS		STISITE / PLAST	TICITY	_
CP= C	Cep Pen		Bulk Sam tre Ölçüm		N : N : N :	0 - 2	Yumuşak/Ver muşak / Soft ta Katı / Medi	ry Soft	N : 0 - 4 N :5 - 10 N :11 - 30	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace of F Az / Low Plastic Orta / Medium Plas	Plasticity	
. ocke			Enginee	r	N:		tı / Stiff k Katı / Ven/	Stiff	N :31 - 50	Siki / Dense	PI :20 - 40%	Yüksek / High Plas		



Test Pit-7

fuc	en .		1	-	/ Expert T			İş	veren / Cl	ient	Proje /Project					
		SIAL		VİRLİK VI	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik						
						ARAŞ	TIRMA	ÇUKU	RU LOG	U / TEST PIT LOG						
ukuı	r No / <sup>-</sup>	Test Pit	No				TP-8			Sayfa No / Page No			1			
ütea	ahhit /	Contrac	ctor							YAS Derinliği / Grour	nd water level	level (m)				
ukur Yeri / Pit Location										Çukur Derinliği / Dep	th of the Test Pit	ne Test Pit (m) 2,00				
arih	/ Date	!			19.10.20	11				Kot / Elevation	(m)					
azı İ	Metodi	u / Exca	vation N	/lethod	by Excav	ator				Koordinatlar / Coordi	nates	N	4.313.050			
Makine / Machine Hidromek 220LC										rtoordinatiai 7 000rdii	nates	Е	679.138			
Derinlik / Depth	Derinlik / Depth Numune No / Sample No Numune Derinligi / Depth of Sample Numune Türü / Type of Sample				Cemin Profile   Zemin Profile   Zemin Cinsi					Zemin Tanımlam	ası / Description	of the Soil				
(m)		(m)		Cinsi / Type	Sonuç / Result											
					(kg/cm <sup>2</sup> )	الله لله د لله	TS									
						vhr vhr										
1																
							GW-GM									
	S-1		D													
3																
ļ																
arkle		-vole-	tion:						Det Cara	üm / Plan View:	Azimut / Az	rimuth:				
KIEانږ	arria / E	Explana	uOH:						USI GORUN	um / Plan View:			<del></del>			
											Α					
										D		В	(m			
											С		Ι .			
													•=			
					1			2C 1 41	N DADA	ACTOC! CD! / OO!!	(m)					
(IS/	ALTMA	ALAR /	ABBRE\	/IATION	İNCE	DANELİ / I	FINE GRA			METRELERÎ / SOIL LÎ / COARSE GRAINS		S STISITE / PLA	STICITY			
	orbo M-	imune '	Dulk Carr	nle	N:		'umuşak/Ver		N:0-4	Çok Gevşek/Very Loose	PI: 1 - 5%	Çok Az / Trace				
			Bulk Sam		N:	3 - 4 Yur	nuşak / Soft		N :5 - 10	Gevşek / Loose	PI :5 - 10%	Az / Low Plastic				
		netromet trometer	re Ölçüm	nü /	N:		a Katı / Medi	um	N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium P				
. 5110			Enginee	r	N : N :		ı / Stiff k Katı / Very	Stiff	N :31 - 50 N :> 50	Siki / Dense Cok Siki / Very Dense	PI :20 - 40% PI :>40%	Yüksek / High P				
	iviU		9100		N:	16 - 30 Çol	∖ ∧aii / very	JUII	IN .~ 5U	Çok Sıkı / Very Dense	F1.24U%	Çok Yüksek / Very High Plastic				



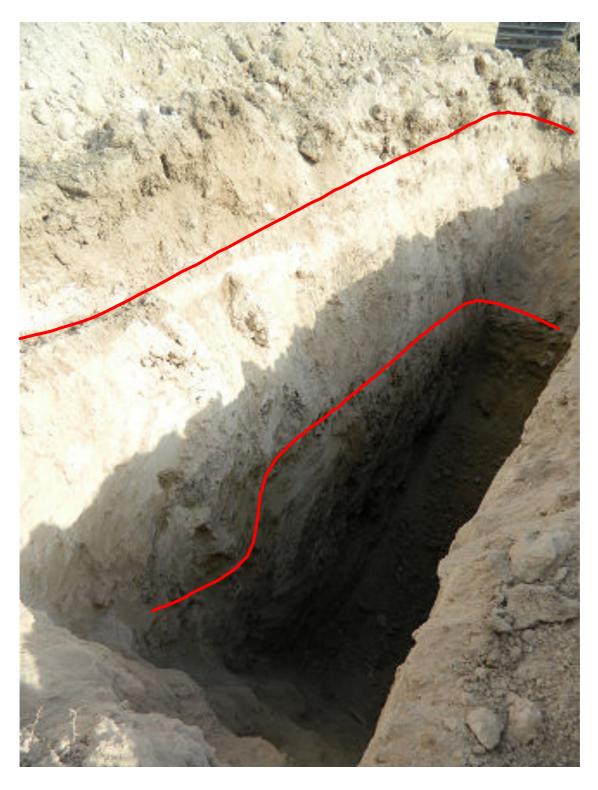
Test Pit-8

		T 1	Hzm	an Ekip /	Evnort 1	Foam		ie	veren / Cli	ont	1	Proje /Projec		
Tugi	RO	SIAL	FUGI	RO SIAL` VIRLIK VE	YERBİLİM	1LERİ			oza Maden			Proje /Projec		
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG	!			
Çukur	No / T	Test Pit	No				TP-9			Sayfa No / Page No			1	
Mütea	hhit /	Contrac	tor							YAS Derinliği / Groun	d water level	(m)		
Çukur	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	2,70	
Tarih /	/ Date				19.10.20	11				Kot / Elevation		(m)		
Kazı N	∕letodu	ı / Exca	vation M	1ethod	by Excav	ator				Koordinatlar / Coordir	nates	N	4.313.09	95
Makin	e / Ma	achine			Hidromel	< 220LC	1			Tito amada 7 Costan		Е	679.352	2
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	ı Test Ö	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class	Zemin Tanımlaması / Description of the Soil						
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
- 1 - 1 2 3	S-1		D			# # # # # # # # # # # # # # # # # # #	TS ML							
- 4 - - - 5 - -														
	ma / E	Explanat	tion:						Üst Görün	üm / Plan View:	Azimut / Azi	imuth:		
										D	A C (m)	В		(m)
KISA	ALTMA	ALAR / A	ABBREV	/IATION	inor	DANELLA	EINE ODA			METRELERÍ / SOIL			STICITY	
CP= C	ep Pen t Penet	netromet trometer	Bulk Sam re Ölçüm Engineel	nü /	INCE DANELİ / FINE GRAINS   IRİ DAN						PLAS PI: 1 - 5% PI: 5 - 10% PI: 10 - 20% PI: 20 - 40% PI:>40%	CTÍSÍTE / PLA  Çok Az / Trace of Az / Low Plastic Orta / Medium P Yüksek / High Pl Çok Yüksek / Ve	of Plasticity Plastic	



Test Pit-9

	<del></del>	Uzman Ekip / Expert Team							veren / Cli	ent	Proje /Project					
-Tug	RD	SIAL	FUG	RO SIAL` VIRLIK VE	YERBİLİN	//LERİ			oza Maden							
						ARAS	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG						
Çukuı	r No / T	Test Pit	No				TP-10			Sayfa No / Page No			1			
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)				
Çukuı	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	3,10			
Tarih	/ Date				19.10.20	11				Kot / Elevation		(m)				
Kazı I	Metodu	u / Exca	avation M	/lethod	by Excav	rator				Koordinatlar / Coordin	nates	N	4.313.210	)		
Makin	ne / Ma	chine		,	Hidrome	k 220LC				reoraniation / coordin	lates	Е	679.072			
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlaması / Description of the Soil						
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)											
- 0,40 m	] 1					1- 12- 12- - 12	TS									
- - 1 - 1,40 n																
- 1,40 m																
- - - 3																
3,10 m																
- <b>4</b>																
-																
- 5 - -																
-																
- 6 Acıkla	ama / F	Explana	tion:			<u> </u>			Üst Görün	üm / Plan View:	Azimut / Azi	imuth:				
<b>A</b> YIKIC	ailid / E	-лріана	uOII.						osi Gorun	D	A C (m)	В	(n	m)		
KIC	ΔΙ ΤΝΑ/	ALAP /		/IATION				ZEMİ	N PARAN	METRELERİ / SOIL						
			Bulk Sam		N:		/umuşak/Ver		N:0-4		PI : 1 - 5%	Çok Az / Trace o	of Plasticity			
CP= C	ep Per		tre Ölçüm		N : N : N :	5 - 8 Ort	muşak / Soft a Katı / Medi tı / Stiff	ium	N :5 - 10 N :11 - 30 N :31 - 50	Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense	PI :5 - 10% PI :10 - 20% PI :20 - 40%	Az / Low Plastic Orta / Medium P Yüksek / High Pl	lastic			
Mühendis / Engineer					N:	16 - 30 Col	k Katı / Very	Stiff	N :> 50	Çok Sıkı / Very Dense		Çok Yüksek / Ve				



Test Pit-10

Uzman Eki					Expert T	eam		İş	veren / Cli	ent	Proje /Project					
Tug		SIAL		RO SİAL` VİRLİK VE LTD.	MÜHEN			K	oza Maden	cilik						
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG						
Çukuı	No / T	Test Pit	No				TP-11			Sayfa No / Page No			1			
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)	4,00			
Çukuı	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept	th of the Test Pit	(m)	4,00			
Tarih	/ Date				17.10.20	11				Kot / Elevation		(m)				
			vation M	/lethod	by Excav					Koordinatlar / Coordir	N 4.313.008					
Makin	ie / Ma	chine			Hidromel	( 220LC						E	678.277			
Derinlik / Depth	Numune No / Sample No Numune Derinligi / Depth of Sample Numune Türü / Type of Sample			Yerinde Insitu	Test	Zemin Profile Soil Profile Zemin Cir				Zemin Tanımlaması / Description of the Soil						
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm <sup>2</sup> )	\ \ <del>\</del> \\										
=						ر بلر بلر بلر بلر	TS									
- -1 -	S-1		D				СН									
- - -3 - -	S-2		D				МН									
-																
-																
-																
- 5																
-																
_																
-																
- 6									1							
Açıkla	Açıklama / Explanation:								Üst Görün	üm / Plan View:	Azimut / Azi	B	(m)			
								7FMi	Ν ΡΔΡΔΙ	METRELERİ / SOIL	(m)	3				
KISA	ALTM/	ALAR / /	ABBRE\	/IATION	İNCE	DANELİ / F	INE GRA			LÍ / COARSE GRAINS		TISITE / PLAS	STICITY			
D = To	orba Nu	ımune /	Bulk Sam	nple	N:	0 - 2 Ç.Y	umuşak/Ver	y Soft	N:0-4	Çok Gevşek/Very Loose	PI : 1 - 5%	Çok Az / Trace o				
CP= C	ep Per		re Ölçüm		N : N : N :	5 - 8 Orta	nuşak / Soft a Katı / Medi ı / Stiff	um	N :5 - 10 N :11 - 30 N :31 - 50	Orta Sıkı / Medium	PI :5 - 10% PI :10 - 20% PI :20 - 40%	Az / Low Plastic Orta / Medium Pl				
			Enginee	r	IN .		Katı / Ven/		N :31 - 50	Cok Siki / Very Dense	PI :20 - 40%	Yüksek / High Pla				



Test Pit-11

			<del></del>								ı	<del></del>		
-Fug	RD (	SIAL	FUG	i <b>an Ekip</b> RO SİAL Y VİRLİK VE I TD	YERBİLİM	/ILERİ			<b>sveren / CI</b> loza Maden			Proje /Projec	:t	
			<b>!</b>		···		TIRMA	CHKH	RILLOG	U / TEST PIT LOG				_
Cuku	No / T	Test Pit	No				TP-12	ÇOITO	110 200	Sayfa No / Page No			1	_
_		Contrac									d water level	(m)		_
		Pit Loc								YAS Derinliği / Groun Çukur Derinliği / Dept		(m)	2,35	_
-	/ Date				17.10.20	11				Kot / Elevation	in or the Test Fit	(m)	,	_
Kazı I	Vetodi	ı / Exca	vation M	/lethod	by Excav							N	4.312.898	_
Makin	ne / Ma	chine			Hidrome	k 220LC	-			Koordinatlar / Coordin	nates	E	678.045	_
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
						$\frac{4}{4}$	TS							
- -1 - - -	S-1		D				SM							
- 3						विद्यास								
- - - 5 -														
- 6 Açıkla	ama / E	Explana	tion:						Üst Görün	ıüm / Plan View:	Azimut / Azi	imuth:		_
										D	A C (m)	В	(m)	
KISA	ALTM!	LAR /	ABBRF\	/IATION						METRELERI / SOIL				_
					INCE N:	DANELİ / F			IRI DANE N:0-4	LÍ / COARSE GRAINS		Cok Az / Trace		
CP= C	ep Pen		Bulk Sam tre Ölçüm		N: N: N: N:	3 - 4 Yum 5 - 8 Orta	'umuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff		N:0-4 N:5-10 N:11-30 N:31-50	Orta Sıkı / Medium	PI : 1 - 5% PI :5 - 10% PI :10 - 20% PI :20 - 40%	Çok Az / Trace of Az / Low Plastic Orta / Medium P Yüksek / High Pl	Plastic	
			Enginee	r	N:		Katı / Very	Stiff	N :> 50		PI :>40%	Çok Yüksek / Ve		



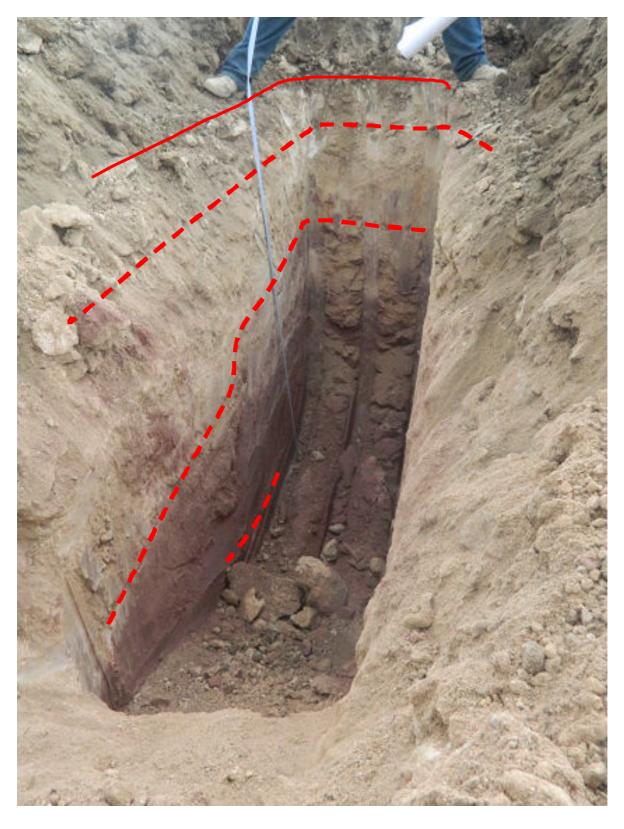
Test Pit-12

	<u> </u>		Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ent		Proje /Projec	t
Tue		SIAL		VİRLİK VE	YERBİLİM E MÜHENI . ŞTİ.			K	oza Maden	cilik			
			,			ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG			
Çukuı	No /	Test Pit	No				TP-13			Sayfa No / Page No			1
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)	
Çukuı	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept	th of the Test Pit	(m)	2,75
Tarih	/ Date				17.10.20°					Kot / Elevation		(m)	
			vation M	lethod	by Excav					Koordinatlar / Coordir	nates	N	4.312.832
Makin	e / Ma				Hidromek	220LC						Е	678.002
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
-						ب بلہ بلہ بلہ	TS						
-1 -1 -2 -3 -3 -4 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5	S-1		D				SM						
- - - 6 Açıkla	ama / E	Explana	tion:						Üst Görüni	üm / Plan View: D	Azimut / Azi A C	imuth:	(m)
											(m)		
KIS	AI TM7	ALAR /	ABBRE\	/IATION						METRELERİ / SOIL	PARAMETERS		
INIO/	¬∟ i IVI <i>}</i> -		יסטעבע	IZTION		DANELI / F				LÍ / COARSE GRAINS		TISITE / PLAS	
D = To	orba Nu	mune / I	Bulk Sam	ıple	N: N:		umuşak/Ver nuşak / Soft		N : 0 - 4 N :5 - 10		PI : 1 - 5% PI :5 - 10%	Çok Az / Trace of Az / Low Plastic	T Plasticity
			re Ölçüm	ıü /	N:	5 - 8 Orta	a Katı / Medi		N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium Pl	
Pocke		trometer	Engineer	r	N:		/ Stiff	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pla	



Test Pit-13

	=	1	Uzm	an Ekip	Expert 1	ream		İş	veren / Cli	ient		Proje /Project	<u> </u>
Tue		SIAL	FUG	RO SIAL` VIRLIK VE	YERBİLİM	/ILERİ			oza Maden				
						ARAŞ	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
Çuku	r No / T	Test Pit	No				TP-14	•		Sayfa No / Page No			1
-		Contrac								YAS Derinliği / Groun	d water level	(m)	
Çuku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	2,20
Tarih	/ Date				17.10ç20	)11				Kot / Elevation		(m)	
Kazı	Metodu	ı / Exca	avation N	/lethod	by Excav	rator				Koordinatlar / Coordin	nates	N	4.312.834
Makir	ne / Ma	chine			Hidromel	k 220LC				Roordinatial / Coordin	iates	E	678.235
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
=						+ 4+ 4+ 4+ 4+ 4+	TS						
- 1	S-1		D				SM						
1,20 r - - - 2,10 r													
- - -3 -													
- 4 													
- - 5 -													
- - 6													
Açıkla	ama / E	Explana	tion:						Üst Görün	üm / Plan View:	Azimut / Azi	imuth:B	(m)
100	A 1		ADDE =	//AT:C::				ZEMİ	N PARAN	METRELERÍ / SOIL		3	
KIS	AL I MA	ALAR / /	ABBRE\	/IATION	İNCE	DANELİ / F	FINE GRA		IRI DANE	LÍ / COARSE GRAINS		STISITE / PLAS	STICITY
			Bulk San		N : N : N :	3 - 4 Yun	'umuşak/Ver nuşak / Soft a Katı / Medi		N : 0 - 4 N :5 - 10 N :11 - 30	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace of Az / Low Plastic Orta / Medium Pla	
	t Penet	trometer			N:	9 - 15 Kat	ı / Stiff		N :31 - 50	Sıkı / Dense	PI :20 - 40%	Yüksek / High Pla	astic
i	Mü	nendis /	Enginee	r	I N	16 - 30 Cok	Katı / Verv	Stiff	N ·> 50	Cok Siki / Very Dense	PI ·>40%	Cok Yüksek / Ver	v High Plactic



Test Pit-14

-fug	DD.		Uzm	an Ekip	Expert 1	eam		İş	veren / Cli	ent		Proje /Projec	ct
		SIAL		VİRLİK VE	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik			
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG	•		
Çuku	r No / T	Test Pit	No				TP-15			Sayfa No / Page No			1
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Grour	id water level	(m)	
Çuku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep		(m)	4,20
Γarih	/ Date				18.10.20	11				Kot / Elevation		(m)	
(azı l	Metodu	ı / Exca	vation N	1ethod	by Excav	ator						N	4.312.957
Лаkir	ne / Ma	chine			Hidromel	< 220LC				Koordinatlar / Coordi	nates	Е	678.505
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
- <del></del>							TS						
- 1 -	S-1		D				SM						
2													
. 4	S-2		D				СН						
6													
- 6 Açıkla	ama / E	Explana	tion:	<u> </u>	l		<u> </u>		Üst Görün	üm / Plan View:	Azimut / Az	rimuth:	
										D	A C (m)	В	(m)
KIS	ALTM/	ALAR / /	ABBRE\	/IATION	1					METRELERÍ / SOIL			0710171
D = To	orba Nu Cep Per	imune / netromet trometer	Bulk San	nple nü /	N: N: N: N:	3 - 4 Yun 5 - 8 Orta 9 - 15 Kat	iumuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff x Katı / Very	y Soft ium	IRI DANE  N: 0 - 4  N: 5 - 10  N: 11 - 30  N: 31 - 50  N: > 50	Lİ / COARSE GRAINS Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PLAS PI: 1 - 5% PI:5 - 10% PI:10 - 20% PI:20 - 40% PI:>40%	Çok Az / Trace Az / Low Plastic Orta / Medium F Yüksek / High P Çok Yüksek / V	of Plasticity : Plastic



Test Pit-15

											1			_
- Tue	RO (	SIAL	FUG	RO SİAL` VİRLİK VE	<b>/ Expert T</b> YERBİLİM E MÜHEN . ŞTİ.	1LERİ			<b>sveren / Cli</b> oza Maden			Proje /Projec	:t	
					•	ARA!	STIRMA	сики	RU LOGI	U / TEST PIT LOG				_
Cuku	· No / T	Test Pit	No				TP-16	30.10		Sayfa No / Page No			1	_
-		Contrac								YAS Derinliği / Groun	d water level	(m)	-	_
		Pit Loc								Çukur Derinliği / Dept		(m)	1,80	_
Tarih	/ Date				17.10.20	11				Kot / Elevation	ar or the restric	(m)	·	_
Kazı I	Metodi	ı / Exca	avation M	/lethod	by Excav	ator	-					N	4.312.830	_
Makir	ne / Ma	chine			Hidromel	x 220LC				Koordinatlar / Coordin	nates	E	678.338	_
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / u Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description (	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
- - -1 -						14 H	TS							
- 2 - - 3 -														
- 4 5														
- - - - 6	ama / F	Explana	tion						lüct Görün	üm / Plan View:	Azimut / Az	imuth:		
<i>r</i> -ryiNid	а / Е	-Apialid	don.						Ost Gorull	D	A C (m)	B	(m)	
KIS	ALTM/	ALAR /	ABBRE\	/IATION						METRELERÍ / SOIL				
						DANELI /				LÍ / COARSE GRAINS		Cok A7 / Trace of		_
D = To	orba Nu	imune / I	Bulk Sam	ıple	N : N :		Yumuşak/Ver muşak / Soft		N : 0 - 4 N :5 - 10	Çok Gevşek/Very Loose Gevşek / Loose	PI : 1 - 5% PI :5 - 10%	Çok Az / Trace of Az / Low Plastic		
			tre Ölçüm	ıü /	N:	5 - 8 Ort	ta Katı / Medi		N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium P	Plastic	
LOCKE		trometer	Enginee		N:		tı / Stiff k Katı / Ven/	Ctiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pl		



Test Pit-16

Ţu G	RO	SIAL	FUG	RO SİAL '	<b>Expert T</b> YERBİLİM MÜHENI	LERİ			veren / Cli oza Maden			Proje /Proje	ct
			wooA		ŞTİ.								
						ARAS		ÇUKU	RU LOGI	J / TEST PIT LOG			
		Γest Pit					TP-17			Sayfa No / Page No			1
		Contrac								YAS Derinliği / Grour	id water level	(m)	
		Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	0,90
	/ Date		4:	4 - 41l	18.10.20					Kot / Elevation		(m)	4.040.50
			vation M	rietnoa	by Excav					Koordinatlar / Coordi	nates	N	4.312.52
akır	ne / Ma				Hidromek	220LC						Е	678.654
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
						\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	TS						
3													
ļ													
çıkla	ama / E	xplana	tion:						Üst Görün	üm / Plan View:	Azimut / Az	imuth:	
,		·u							2. 20.011		A		<b>_</b>
											П		
										D		В	1)
											С		$\perp$
										+	(m)		
								ZFMİ	N PARAN	METRELERİ / SOIL		S	
(IS	ALTM/	ALAR / /	ABBRE\	/IATION	İNCE	DANELİ /	FINE GRA			LÍ / COARSE GRAINS		STISITE / PLA	ASTICITY
= To	orba Nu	ımune / l	Bulk Sam	ple	N:	0 - 2 Ç.Y	'umuşak/Ver	y Soft	N:0-4	Çok Gevşek/Very Loose	PI : 1 - 5%	Çok Az / Trace	of Plasticity
			re Ölçüm		N : N :		nuşak / Soft a Katı / Medi		N :5 - 10 N :11 - 30	Gevşek / Loose Orta Sıkı / Medium	PI :5 - 10%	Az / Low Plastic	
		rometer	ie Oiçum	iu /	N: N:		a Katı / Medi ı / Stiff		N :11 - 30 N :31 - 50	Orta Siki / Medium Siki / Dense	PI :10 - 20% PI :20 - 40%	Orta / Medium I Yüksek / High F	
	8.47	la a sa all'a - /	Enginee	_	1 1		Katı / Very		N :> 50	Çok Sıkı / Very Dense	PI :>40%	Çok Yüksek / V	



Test Pit-17

fuc	Pn -			-	/ Expert T			İş	şveren / Cl	ient		Proje /Proje	ct	_
100		SIAL		VİRLİK VI	YERBİLİM E MÜHENI . ŞTİ.			К	oza Maden	cilik				
						ARAS	ŞTIRMA	ÇUKU	RU LOG	U / TEST PIT LOG	ì			
ukuı	r No / ٦	est Pit	No				TP-18			Sayfa No / Page No		_	1	
ütea	ahhit /	Contrac	tor							YAS Derinliği / Grou	nd water level	(m)		
ukuı	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	0,70	
arih	/ Date				18.10.20	11				Kot / Elevation		(m)		
azı I	Metodu	ı / Exca	vation N	/lethod	by Excav					Koordinatlar / Coordi	nates	N	4.312.433	3
akir	ne / Ma	chine		1	Hidromek	220LC		1				E	678.896	i
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description o	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
1 2 2 3 3							TS							
6 .çıkla	ama / E	xplana	tion:							üm / Plan View:	Azimut / Az A C (m)	В	(r	m)
KIS	ALTMA	LAR /	ABBRE\	/IATION	INICE	DANELİ /	EINE CD4			METRELERÎ / SOII LÎ / COARSE GRAINS		STISITE / PLA	STICITY	_
P= C	Cep Pen t Penet	etromet	Bulk San re Ölçüm Enginee	nü /	N: N: N: N:	0 - 2 Ç.\\ 3 - 4 Yu 5 - 8 Ort 9 - 15 Ka	Yumuşak/Ver muşak / Soft ta Katı / Medi tı / Stiff k Katı / Very	y Soft ium	N:0-4 N:5-10 N:11-30 N:31-50 N:>50	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PI: 1 - 5% PI: 5 - 10% PI: 10 - 20% PI: 20 - 40% PI: >40%	Çok Az / Trace Az / Low Plastic Orta / Medium F Yüksek / High F Çok Yüksek / V	of Plasticity Plastic	



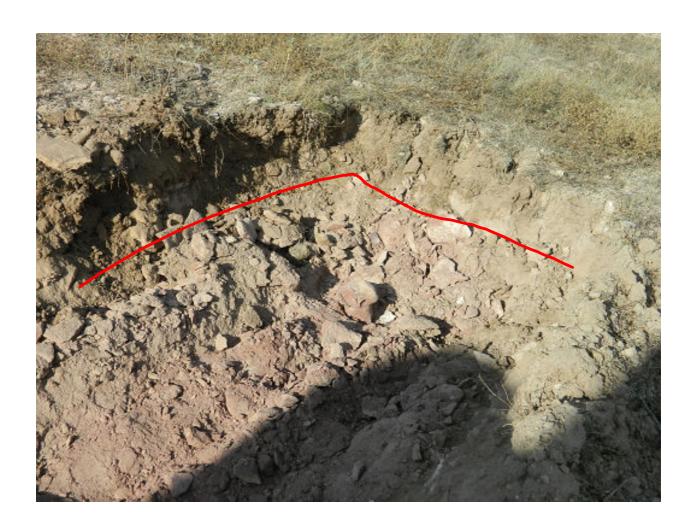
Test Pit-18

			11	an Elda	/ Francout 7	T					Ī	Dunia (Dunia		
-Fug	RD (	SIAL	FUG	RO SIAL` VIRLIK VE	<b>/ Expert T</b> YERBİLİM E MÜHENI . ŞTİ.	1LERİ			<b>sveren / Cli</b> oza Maden			Proje /Projed	:t	
					<del></del>	ΛΡΛ <b>ς</b>	TIDMA	CHKH	BILLOGI	J / TEST PIT LOG				
Culan	· No / T	Test Pit	No		<u> </u>	ANAS	TP-19	ÇUNU	NO LOGI	Sayfa No / Page No		1	1	
_		Contrac					11-13			<u> </u>		(m)		
		Pit Loc								YAS Derinliği / Groun		(m)	2,00	0
-	/ Date		ation		18.10.20°	11				Çukur Derinliği / Dept	n of the Test Pit	(m)		
			vation M	/lethod	by Excav					Kot / Elevation		N N	4.312.	154
	ne / Ma				Hidromek					Koordinatlar / Coordir	nates	E	679.1	
th	Numune No / Sample No	nliği / ıple	Numune Türü / Type of Sample	Yerinde Insitu	Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o			
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	V- V-	TO							
-	l					ر بلہ بلہ بلہ بلہ	TS							
- 0,40 r - - 1	n ————————————————————————————————————													
- - -3														
- - 4 -														
- 5 - - -														
	ıma / E	Explana	tion:						Üst Görüni	üm / Plan View:	Azimut / Az	imuth:		
		-								D	A C (m)	В		(m)
KISA	4LTMA	ALAR / /	ABBRE\	/IATION	inor	DANELL	CINIC OD 4			METRELERÍ / SOIL			CTICITY	
D = To	orba Nu	ımune / l	Bulk Sam	nple	N: N: N:	3 - 4 Yur	FINE GRA /umuşak/Ver muşak / Soft :a Katı / Medi	y Soft	N: 0 - 4 N:5 - 10 N:11 - 30	Gevşek / Loose	PLAS PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace Az / Low Plastic	of Plasticity	
	t Penet	trometer			N:		tı / Stiff		N :31 - 50		PI :20 - 40%	Yüksek / High P		_



Test Pit-19

Ţue	RD (	SIAL	FUG	RO SİAL` VİRLİK VE	<b>/ Expert T</b> YERBİLİM E MÜHEN . ŞTİ.	ILERI			<b>sveren / Cli</b> oza Maden			Proje /Projec	t	
					•	AR/	ASTIRMA	сики	RU LOGI	U / TEST PIT LOG				
Cuku	· No / -	Test Pit	No				TP-20	30.10		Sayfa No / Page No			1	
-		Contrac								YAS Derinliği / Groun	d water level	(m)		
		Pit Loc			<u> </u>					Çukur Derinliği / Dept		(m)	0,90	
Tarih	/ Date				18.10.20	11				Kot / Elevation	ar or the restric	(m)		
Kazı I	Metodu	ı / Exca	avation N	/lethod	by Excav	ator						N	4.312.276	<del></del>
Makir	ie / Ma	chine			Hidromel	k 220LC				Koordinatlar / Coordin	nates	Е	679.369	
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / u Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	50, 50	s,							
- -		,					TS							
- 1														
- 2 - 2 3 4 5 5	ma / F	-volana	tion						lüst Görün	üm / Plan Views	Azimut / Az	imuth:		
Açıkla	ama / E	Explana	tion:						Üst Görün	üm / Plan View:	Azimut / Az  A  C  (m)	imuth:	(n	n)
KIS	ALTM#	ALAR /	ABBRF\	/IATION						METRELERI / SOIL				
					INCE N:		/ FINE GRA		IRI DANE N:0-4	LÍ / COARSE GRAINS	PLAS PI : 1 - 5%	Cok Az / Trace		
			Bulk Sam		N:	3 - 4	Ç.Yumuşak/Ver Yumuşak / Soft		N :5 - 10	-	PI :5 - 10%	Çok Az / Trace		
		netromet trometer	tre Ölçüm	ıü /	N : N :		Orta Katı / Medi Katı / Stiff	ium	N :11 - 30 N :31 - 50	Orta Sıkı / Medium Sıkı / Dense	PI :10 - 20% PI :20 - 40%	Orta / Medium F Yüksek / High P		
			Enginee	r	NI .		Cok Katı / Verv	Stiff	N :> 50	Cok Siki / Very Dense	PI:>40%	Cok Yüksek / V		



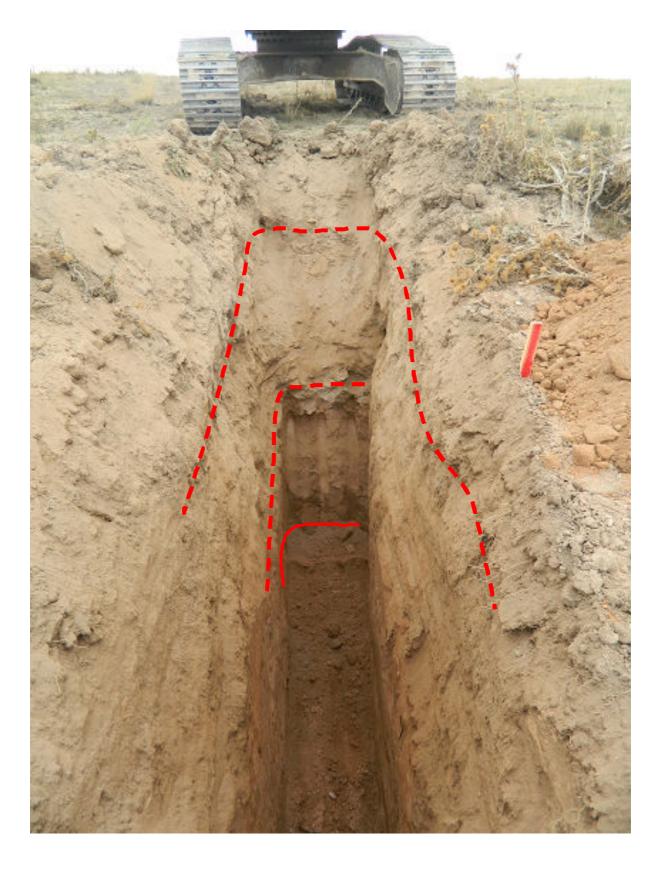
Test Pit-20

		1	Uzm	an Ekip	/ Expert T	eam		İş	şveren / Cli	ient		Proje /Project	1
Tue		SIAL	FUG	RO SİAL` VİRLİK VE	YERBİLİM	1LERİ			oza Maden				
						ARAS	TIRMA	ÇUKU	RU LOG	U / TEST PIT LOG			
Çukuı	r No / ·	Test Pit	No				TP-21			Sayfa No / Page No			1
_		Contrac								YAS Derinliği / Groun	d water level	(m)	
Çukuı	r Yeri /	Pit Loc	ation				-			Çukur Derinliği / Dept		(m)	2,10
Tarih	/ Date	!			18.10.20	11				Kot / Elevation		(m)	
Kazı I	Metod	u / Exca	avation M	/lethod	by Excav	ator				Koordinatlar / Coordin	nates	N	4.312.474
Makin	ne / Ma	chine			Hidromel	< 220LC				Roordinatial / Coordin	iates	E	679.196
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
0,30 n	l n					الماريد) معلى جولي المسارية	TS						
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- 6													
	ama / E	xplana	tion:						Üst Görün	üm / Plan View:	Azimut / Azi	muth:	<b>→</b>
İ										D	A C	В	(m)
									<u>L</u>		(m)		
KISA	ALTM/	ALAR / /	ABBRE\	/IATION						METRELERÍ / SOIL			
						DANELİ / F			IRI DANE N:0-4	LÍ / COARSE GRAINS	PLAS PI : 1 - 5%	Cok Az / Trace of	
D = To	orba Nu	ımune /	Bulk Sam	ıple	N : N :		'umuşak/Ver nuşak / Soft		N : 0 - 4 N :5 - 10	Çok Gevşek/Very Loose Gevşek / Loose	PI : 1 - 5% PI :5 - 10%	Çok Az / Trace of Az / Low Plastic	ı idəliciliy
		netromet trometer	tre Ölçüm	ıü /	N :	5 - 8 Orta	a Katı / Medi		N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium Pla	
. ocke			Enginee	r	N:		ı / Stiff « Katı / Ven/	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pla	



Test Pit-21

fuc				-	Expert T			İş	şveren / Cli	ent		Proje /Proje	ct	
<b>V</b>		SIAL		VİRLİK VE	YERBİLİM E MÜHENI ŞTİ.			K	oza Maden	cilik				
						ARAŞ	TIRMA	ÇUKU	RU LOG	J / TEST PIT LOG				
uku	r No / ٦	Γest Pit	No				TP-22			Sayfa No / Page No			1	
lütea	ahhit / (	Contrac	tor							YAS Derinliği / Grour	nd water level	(m)		
uku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	3,20	
arih	/ Date				18.10.20°	11				Kot / Elevation		(m)		
azı l	Metodu	ı / Exca	vation M	1ethod	by Excav					Koordinatlar / Coordi	nates	N	4.312.74	13
akir	ne / Ma	chine			Hidromek	220LC	,					Е	678.947	7
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil		
(m)		(m)		Cinsi / Type	Result (kg/cm <sup>2</sup> )									
1						<b>L</b> , L	TS							
2	S-1		D				МН							
			D				МН							
4														
5														
6 Cıkla	ame / E	vnlana	tion:						Üet Görün	üm / Plan View:	Azimut / Az	rimuth:		
ÇIKla	ama / E	Explana	uon:							D	A C (m)	В		(m)
KIS	ALTMA	ALAR / /	ABBREV	/IATION	ivor	DANELL	INIE OD:			METRELERÍ / SOIL			VOTIOITY	_
P= C	Cep Pen et Penet	netromet	Bulk Sam re Ölçüm Enginee	ıü /	N: N: N: N:	3 - 4 Yun 5 - 8 Orta 9 - 15 Kat	INE GRA umuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff : Katı / Very	y Soft ium	N: 0 - 4 N: 5 - 10 N: 11 - 30 N: 31 - 50 N: 50	Lİ / COARSE GRAINS  Çok Gevşek/Very Loose  Gevşek / Loose  Orta Sıkı / Medium  Sıkı / Dense  Çok Sıkı / Very Dense	PLAS PI: 1 - 5% PI: 5 - 10% PI: 10 - 20% PI: 20 - 40% PI: >40%	Çok Az / Trace Az / Low Plastic Orta / Medium F Yüksek / High F Çok Yüksek / V	of Plasticity  Plastic  Plastic	



Test Pit-22

-fue	<u> </u>	  -	Uzm	an Ekip	Expert 1	eam		İş	veren / Cli	ent	<u> </u>	Proje /Projec	ot
		SIAL		VİRLİK VI	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik			
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG	•		
Çuku	r No / T	Test Pit	No				TP-23			Sayfa No / Page No			1
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Grour	nd water level	(m)	
Çuku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep		(m)	4,40
Γarih	/ Date				19.10.20	11				Kot / Elevation		(m)	
(azı l	Metodu	ı / Exca	vation N	1ethod	by Excav	ator						N	4.312.977
Лаkir	ne / Ma	chine			Hidromel	220LC				Koordinatlar / Coordi	nates	Е	679.695
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / I Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	\ L, \.							
	S-1		D				СН						
6									Is				
		Explana		/IATION	İNCE	DANELÍ / F	FINE GRA		N PARAM IRI DANE	üm / Plan View:  D  HETRELERİ / SOIL Lİ / COARSE GRAINS	PLAS	В	(m)
CP= C	Cep Per t Penet	netromet trometer	Bulk San re Ölçüm Enginee	ıü /	N: N: N: N: N:	3 - 4 Yun 5 - 8 Orta 9 - 15 Kati	umuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff : Katı / Very	ium	N:0-4 N:5-10 N:11-30 N:31-50 N:>50	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PI: 1 - 5% PI:5 - 10% PI:10 - 20% PI:20 - 40% PI:>40%	Çok Az / Trace Az / Low Plastic Orta / Medium F Yüksek / High P Çok Yüksek / Ve	Plastic lastic



Test Pit-23

-fugro					/ Expert Team İşver					ent	Proje /Project		
		SIAL		VİRLİK VI	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik			
						ARAS	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
ukuı	r No / <sup>-</sup>	Test Pit	No				TP-24			Sayfa No / Page No			1
ütea	ahhit /	Contrac	ctor							YAS Derinliği / Grour	nd water level	(m)	
uku	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	1,80
arih	/ Date				19.10.20	11				Kot / Elevation (m)			
azı l	Metodu	u / Exca	vation N	/lethod	by Excav					Koordinatlar / Coordi	nates	N	4.312.791
akir	ne / Ma	achine		T	Hidromel	220LC						Е	679.554
Derinlik / Depth	Numune No / Sample No Numune Derinliği / Depth of Sample No Numune Türü / Type of Sample   Type of Sample   Zemin Profili / Soil Profile   Zemin Cinsi / Soil Class						Zemin Tanımlaması / Description of the Soil						
(m)		(m)		Cinsi / Type	Result (kg/cm <sup>2</sup> )								
					(kg/ciii )		TS						
1	S-1		D				МН						
2													
3													
4													
,													
i													
	ama / E	xplana	tion:						Üst Görün	üm / Plan View:	Azimut / Az	zimuth:	
											A		Τ
										D		ے	
										_		В	(m
											С		_
											(m)		
(15.	AI TM4	ALAR /	ABBRF\	/IATION						//ETRELERİ / SOIL			
	, «⊏ , IVI <i>)</i> -	12/11\/ /	ייסטועבע	ALION		DANELİ /				LÍ / COARSE GRAINS		STISITE / PLA	
= To	orba Nu	ımune /	Bulk San	nple	N: N:		/umuşak/Ver muşak / Soft		N : 0 - 4 N :5 - 10	Çok Gevşek/Very Loose Gevşek / Loose	PI : 1 - 5% PI :5 - 10%	Çok Az / Trace Az / Low Plastic	
			re Ölçüm	nü /	N:		a Katı / Med		N :5 - 10 N :11 - 30	Orta Sıkı / Medium	PI :5 - 10% PI :10 - 20%	Orta / Medium F	
ocke		trometer			N:		tı / Stiff		N :31 - 50	Sıkı / Dense	PI :20 - 40%	Yüksek / High P	
Mühendis / Engineer					N:	16 - 30 Ço	k Katı / Very	Stiff	N :> 50	Çok Sıkı / Very Dense	PI :>40%	Çok Yüksek / V	ery High Plastic



Test Pit-24

Uzman Ekip					-			İş	veren / Cl	Client Proje /Project				
V		SIAL		VİRLİK VE	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik				
						ARAŞ	TIRMA	ÇUKU	RU LOG	U / TEST PIT LOG				
ukuı	r No / ٦	Γest Pit	No				TP-25			Sayfa No / Page No			1	
ütea	ahhit / (	Contrac	ctor							YAS Derinliği / Grour	nd water level	(m)		
ukuı	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	3,00	
arih	/ Date				19.10.20	11				Kot / Elevation		(m)		
azı İ	Metodu	ı / Exca	vation M	1ethod	by Excav	ator				Koordinatlar / Coordi	nates	N	4.312.50	ე9
akir	ne / Ma	chine			Hidromel	220LC				Roordinatial 7 Goordi	nates	E	679.710	0
Derinlik / Depth	Numune No / Sample No / Sample No / Numune Derinligi / Depth of Sample / Type of Sample / T					Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class	Zemin Tanımlaması / Description of the Soil						
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
						مل مل مل	TS							
	S-1		D			مل جل حل	CL							
1	S-2		D				МН							
2														
1														
5														
ŝ .çıkla	ama / E	Explana	tion:						Üst Görün	üm / Plan View:	Azimut / Az	zimuth:		_
guc					T			7514		D  METRELERÍ / SOIL	A C (m)	В		(m
KISA	ALTMA	LAR / /	ABBREV	/IATION	İNCE	DANELİ / F	FINE GRA			LÍ / COARSE GRAINS		STISITE / PLA	ASTICITY	_
KISALTMALAR / ABBREVIATION  D = Torba Numune / Bulk Sample  CP= Cep Penetrometre Ölçümü / Pocket Penetrometer  Mühendis / Engineer				N: N: N: N:	0 - 2 Ç.Y 3 - 4 Yun 5 - 8 Orta 9 - 15 Kat	rumuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff k Katı / Very	y Soft ium	N: 0 - 4 N: 5 - 10 N: 11 - 30 N: 31 - 50 N: > 50	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PI: 1 - 5% PI:5 - 10% PI:10 - 20% PI:20 - 40% PI:>40%	Çok Az / Trace Az / Low Plastic Orta / Medium I Yüksek / High F Çok Yüksek / V	of Plasticity  C  Plastic  Plastic		

No Picture for Test Pit-25

		-1	Uzm	an Ekip	Expert T	eam		İş	veren / Cli	ent		Proje /Project		
Tue		SIAL		VİRLİK VE	YERBİLİM E MÜHEN ŞTİ.			K	oza Maden	cilik				
					•	ARAS	TIRMA	CUKU	RU LOGI	J / TEST PIT LOG				
Çuku	r No / T	Test Pit	No				TP-26			Sayfa No / Page No			1	
Müteahhit / Contractor										YAS Derinliği / Groun	d water level	(m)		
Çukur Yeri / Pit Location										Çukur Derinliği / Dept		(m)	4,40	
Tarih	/ Date	1			18.10.20	11				Kot / Elevation (m)		(m)		
Kazı I	Metodi	u / Exca	vation N	/lethod	by Excav	ator				Koordinatlar / Coordinates N 4.312			4.312.183	
Makir	ne / Ma				Hidromel	220LC				rtoordinatian / Goordin	iates	E	679.681	
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Zemin Profili / Soil Profile / Zemin Cinsi / Soil Class					Zemin Tanımlaması / Description of the Soil				
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm <sup>2</sup> )									
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-														
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- 1	S-1		D											
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- 6 Acıkla	ama / E	Explana	tion:						Üst Görüni	üm / Plan View:	Azimut / Az	imuth:		
,		. ,									A		<del></del>	
											A			
										D		В	(m)	
										<u> </u>	С		$\perp$	
										<del> </del>	(m)			
NIC.	ΛΙ <b>Τ</b> Α 4 4	ALAD /	VDDDC,	/IATION				ZEMİ	N PARAM	METRELERİ / SOIL		3		
NIS/	~∟ I (VI <i>F</i>	ALAK / /	HDBKE\	MATION		DANELİ / F		INS	İRİ DANEI	.İ / COARSE GRAINS	PLAS	STISITE / PLA		
D = To	orba Nu	ımune / l	Bulk Sam	nple	N:		umuşak/Ver	y Soft	N : 0 - 4 N :5 - 10		PI : 1 - 5%	Çok Az / Trace		
CP= C	ep Per	netromet	re Ölçüm	nü /	N: N:		nuşak / Soft a Katı / Medi	um	N :5 - 10 N :11 - 30		PI :5 - 10% PI :10 - 20%	Az / Low Plastic Orta / Medium F		
Pocket Penetrometer					N :		/ Stiff		N :31 - 50	Sıkı / Dense	PI :20 - 40%	Yüksek / High P		



Test Pit-26

	=	Uzm	an Ekip	/ Expert T	eam		İş	veren / Cl	ent		Proje /Proje	ct		
<b>T</b> UGR	SIAL		VİRLİK VI	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik					
		<u>-</u>			ARAŞ	TIRMA	ÇUKU	RU LOG	J / TEST PIT LOG					
ukur	No / Test	Pit No				TP-27			Sayfa No / Page No			1		
üteal	hhit / Cont	ractor							YAS Derinliği / Grour	nd water level	(m)			
ukur	Yeri / Pit l	ocation							Çukur Derinliği / Dep		(m)	1,50		
arih /	Date			18.10.20	11				Kot / Elevation		(m)			
azı M	letodu / Ex	cavation N	/lethod	by Excav	ator						N	4.311.929		
akine	e / Machin	e		Hidromel	< 220LC				Koordinatlar / Coordi	nates	Е	679.481		
				e Deney / Vernin Profile / Soil Class / Soil Class				Zemin Tanımlaması / Description of the Soil						
(m)	(m	)	Cinsi / Type	Result (kg/cm <sup>2</sup> )										
2 3 3						TS		Top Soil						
6 Açıklar	ma / Expla	nation:						Üst Görün	üm / Plan View:	Azimut / Az	zimuth:	(m		
									<u> </u>	(m)		<u></u>		
KISA	LTMALAR	R / ABBRE\	/IATION	1	DANE: 1::				METRELERÍ / SOIL			OTIOIT:		
= Tor	ba Numun	e / Bulk San	nple	N: N: N:	3 - 4 Yur	FINE GRA 'umuşak/Ver nuşak / Soft a Katı / Medi	y Soft	IRI DANE N:0-4 N:5-10 N:11-30	Lİ / COARSE GRAINS  Çok Gevşek/Very Loose  Gevşek / Loose  Orta Sıkı / Medium	PLAS PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Çok Az / Trace Az / Low Plastic Orta / Medium F	of Plasticity		
	Penetrome			N:		ı / Stiff		N :31 - 50	Siki / Dense	PI :20 - 40%	Yüksek / High F			
	Mühend	is / Enginee	r	N :	16 - 30 Çol	Katı / Very	Stiff	N :> 50	Çok Sıkı / Very Dense	PI :>40%	Çok Yüksek / V	ery High Plastic		

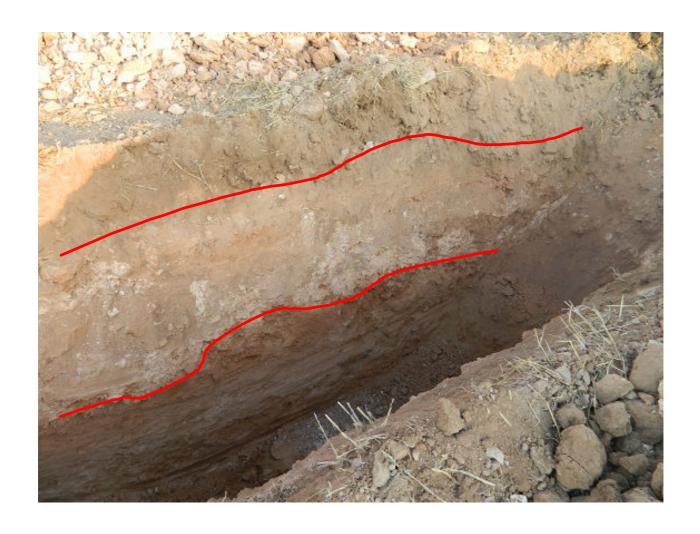


Test Pit-27

	<del></del>	I	Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ent		Proje /Project			
Ţug		SIAL		VİRLİK VE	YERBİLİM E MÜHENI . ŞTİ.			K	oza Maden	cilik					
						ARAŞ	TIRMA	ÇUKU	RU LOGI	J / TEST PIT LOG					
Çukur	No /	Γest Pit	No				TP-28			Sayfa No / Page No			1		
Müteahhit / Contractor										YAS Derinliği / Groun	d water level	(m)			
Çukur Yeri / Pit Location										Çukur Derinliği / Dept	th of the Test Pit	(m)	2,30		
Tarih / Date					18.10.20°					Kot / Elevation		(m)			
Kazı Metodu / Excavation Method					by Excav					Koordinatlar / Coordir	natlar / Coordinates N 4.311.64				
Makine / Machine					Hidromek	220LC				E 680.029					
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Zemin Profili / Soil Class					Zemin Tanımlaması / Description of the Soil					
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \									
-						\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	TS		Top Soil						
0,45 n	n ——					*******									
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-															
-															
- 6															
Açıkla	ima / E	xplana	tion:						Ust Görüni	üm / Plan View:	Azimut / Azi	muth:	<del></del>		
										D	A C (m)	В	(m)		
KISA	ALTM <i>A</i>	ALAR / /	ABBRE\	/IATION						METRELERİ / SOIL					
					INCE N:	0 - 2 Ç.Y	FINE GRA 'umuşak/Ver		IRI DANEI N:0-4	LÍ / COARSE GRAINS  Çok Gevşek/Very Loose	PLAS PI : 1 - 5%	Çok Az / Trace o			
D = To	orba Nu	imune / I	Bulk Sam	ıple	N:	-	nuşak / Soft		N : 0 - 4 N :5 - 10			Az / Low Plastic	л газиону		
			re Ölçüm	ü/	N:		a Katı / Medi	um	N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium P			
Pocket Penetrometer  Mühendis / Engineer					N:		ı / Stiff « Katı / Ven/	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pl			

No Picture for Test Pit-28

		ı	Uzm	an Ekip	/ Expert T	eam		İş	İşveren / Client Proje /Proj				:t	
Ţugi		SIAL	FUG	RO SIAL Y VİRLİK VE	YERBİLİM E MÜHENI . ŞTİ.	ILERİ		K	oza Maden	cilik				
			ļ	-		ARA	ŞTIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG				
Çukur	No /	Test Pit	No	-			TP-29			Sayfa No / Page No			1	
Mütea	ahhit /	Contrac	tor							YAS Derinliği / Groun	d water level	(m)		
Çukur Yeri / Pit Location										Çukur Derinliği / Dept		(m)	3,00	
Tarih	/ Date				18.10.20	11				Kot / Elevation		(m)		
Kazı M	Metodi	ı / Exca	vation M	/lethod	by Excav	ator				Koordinatlar / Coordinates N 4.312.138				
Makin	ie / Ma	chine			Hidromek	220LC				Roordinatial / Coordin	iates	E	680.162	
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample Type of Sample Abelia / Zemin Profili / Soil Class						Zemin Tanımlaması / Description of the Soil					
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
-							TS		Top Soil					
0,40 n						White colored highly weathered rock (Tuff), cemented by washing calcium carbonated water, reasonably hard								
1,00 n n 2						· × · · · · · · · · · · · · · · · · · ·			Redish brown soil	n colored, silty clayey sand wi	th few gravel, sligtly wet	, loose to medium	n dense, residual	
- 6	ma / F	-vnlana	tion:						Üct Görün	üm / Plan View:	Azimut / Azi	imuth:		
AÇIKIR	Açıklama / Explanation:									D	C (m)	В	(m)	
KISA	ALTM/	ALAR /	ABBRE\	/IATION	ince	DANEL	/FINE OB!			METRELERÍ / SOIL			CTICITY	
					INCE N:		/ FINE GRA :Yumuşak/Ver		N:0-4	Lİ / COARSE GRAINS  Çok Gevşek/Very Loose	PLAS PI:1-5%	GTİSİTE / PLAS Çok Az / Trace o		
CP= C	ep Per		Bulk Sam tre Ölçüm		N : N :	3 - 4 Y 5 - 8 O	umuşak / Soft Orta Katı / Medi		N :5 - 10 N :11 - 30	Gevşek / Loose Orta Sıkı / Medium	PI :5 - 10% PI :10 - 20%	Az / Low Plastic Orta / Medium Pl	lastic	
. Jone			Enginee	r	N:		atı / Stiff tok Katı / Verv	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pla		



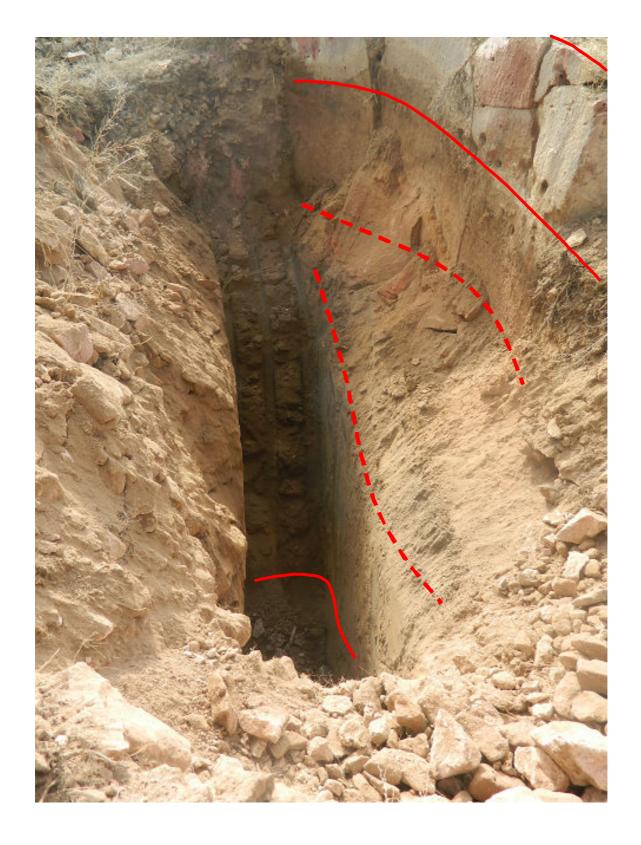
Test Pit-29

	<u> </u>	1	Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ent		Proje /Project	t
-Tug		SIAL			YERBİLİM E MÜHEN ŞTİ.			K	oza Maden	cilik			
						ARAŞ	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
Çukur	No / -	Test Pit	No				TP-31			Sayfa No / Page No			1
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)	4,40
Çukur	Yeri /	Pit Loc	ation							Çukur Derinliği / Dept	th of the Test Pit	(m)	4,60
Tarih	/ Date				19.10.20	11				Kot / Elevation		(m)	
Kazı M	Metodi	u / Exca	vation M	1ethod	by Excav	ator				Koordinatlar / Coordir	nates	N	4.312.681
Makin	e / Ma	chine			Hidromel	220LC	1					Е	680.204
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result								
-					(kg/cm <sup>2</sup> )	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TS						
-						チゲゲ							
- - 1 -													
-							CL						
- 2													
-	S-1		D										
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- 5													
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_													
- 6													
	ıma / E	xplana	tion:					l.	Üst Görün	üm / Plan View:	Azimut / Azi	imuth:	<b>—</b>
										D	A C (m)	В	(m)
KIC	ΔΙ ΤΝ <i>Ι</i> Ι	ALAR /		/IATION						//ETRELERÎ / SOIL			
NIOF	¬∟ i IVI <i>F</i>	ALMR / /	-DDKE\	MATION		DANELİ / I				LÍ / COARSE GRAINS		TISITE / PLAS	
D = To	orba Nu	ımune /	Bulk Sam	ple	N: N:		'umuşak/Ver nuşak / Soft	-	N : 0 - 4 N :5 - 10		PI : 1 - 5% PI :5 - 10%	Çok Az / Trace of Az / Low Plastic	r Plasticity
			re Ölçüm	ıü /	N:		a Katı / Medi		N :11 - 30	Orta Sıkı / Medium	PI :10 - 20%	Orta / Medium Pla	astic
Pocke		trometer	Enginee	r	N:		/ Stiff	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Pla	



Test Pit-31

	=		Uzm	an Ekip	Expert T	eam		İ	şveren / Cl	ient		Proje /Proje	ct
Tugi		SIAL		VİRLİK VI	YERBİLİM E MÜHEN . ŞTİ.			к	oza Mader	ncilik			
						ARAŞ	TIRMA	ÇUKU	RU LOG	U / TEST PIT LOG	ì		
ukur	No / 1	Test Pit	No				TP-32			Sayfa No / Page No			1
ütea	hhit /	Contrac	ctor							YAS Derinliği / Groui	nd water level	(m)	
ukur	Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	5,35
arih .	/ Date				18.10.20	11				Kot / Elevation		(m)	
azı N	Metodu	ı / Exca	vation N	lethod	by Excav	ator				Koordinatlar / Coordi	nates	N	4.312.878
akin	e / Ma	chine			Hidromel	220LC				Troor amadian 7 Goor an		E	678.471
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Insitu	Deney / i Test  Sonuç / Result	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description o	of the Soil	
,,		(111)		Туре	(kg/cm <sup>2</sup> )	TE TE							
						1	TS						
						<u></u>							
ı													
3													
4													
5													
çıkla	ıma / E	Explana	tion:		I		<u> </u>	I	Üst Görün	nüm / Plan View:	Azimut / Azi	imuth:	
											A		_ <b>-</b>
											- •		
										D		В	(m)
										<u> </u>	С		$\perp$
											(m)		
ZIO 4	\   T \ 4 4	N AD 1	V D D D C,	/IATION/				ZEMİ	N PARAI	METRELERİ / SOIL	٠,,	3	
NS/	AL I MA	ALAK / /	HRRKF/	/IATION		DANELİ /		AINS	İRİ DANE	LÍ / COARSE GRAINS	PLAS	TISITE / PLA	
= To	rba Nu	ımune /	Bulk Sam	ple	N:		'umuşak/Ver		N:0-4	Çok Gevşek/Very Loose	PI : 1 - 5%	Çok Az / Trace	
P= C	ep Per	netromet	re Ölçüm	ıü /	N: N:		nuşak / Soft a Katı / Med		N :5 - 10 N :11 - 30	Gevşek / Loose Orta Sıkı / Medium		Az / Low Plastic Orta / Medium F	
		rometer			N :		ı / Stiff		N :31 - 50	Siki / Dense		Yüksek / High P	
	Mü	hendis /	Enginee	r	N:	16 - 30 Çol	k Katı / Very	Stiff	N :> 50	Çok Sıkı / Very Dense	PI :>40%	Çok Yüksek / V	erv High Plastic



Test Pit-32

		1	Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ient		Proje /Project	
Tug		SIAL	FUG	RO SİAL \ VİRLİK VE LTD.	YERBİLİM E MÜHENI	ILERİ			oza Maden				
						ARAS	STIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
Çukuı	No / <sup>-</sup>	Test Pit	No				TP-33			Sayfa No / Page No			1
-		Contrac								YAS Derinliği / Groun	d water level	(m)	
Çukuı	r Yeri /	Pit Loc	ation							Çukur Derinliği / Dept		(m)	4,20
Tarih	/ Date				18.10.20	11				Kot / Elevation		(m)	
Kazı I	Metodu	u / Exca	avation M	/lethod	by Excav	ator				Koordinatlar / Coordin	nates	N	4.312.719
Makin	ne / Ma	achine			Hidromek	( 220LC				Roordinatial / Coordin	iates	E	678.715
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / ı Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)								
-						\dr \dr \dr \dr \dr \dr \dr \dr \dr \dr \dr	TS						
- -1 - - -													
- - - 3 -													
- - 4													
- - - 5 - -													
Açıkla	ıma / E	Explana	tion:							üm / Plan View:	Azimut / Azi	В	(m)
KISA	ALTM/	ALAR / /	ABBRE\	/IATION	1	DA.:=: '	=INI=			METRELERÍ / SOIL			O.T.
					INCE N:	0 - 2 Ç.Y	FINE GRA Yumuşak/Ver		İRİ DANE N∶0-4	Lİ / COARSE GRAINS  Çok Gevşek/Very Loose	PLAS PI : 1 - 5%	Cok Az / Trace of Pla	
CP= C	Cep Per	netromet	Bulk Sam tre Ölçüm		N : N :	3 - 4 Yu	muşak / Soft ta Katı / Medi		N :5 - 10 N :11 - 30	Gevşek / Loose Orta Sıkı / Medium	PI : 1 - 5% PI :5 - 10% PI :10 - 20%	Az / Low Plastic Orta / Medium Plastic	
Pocke		trometer	Enginee	r	N:		tı / Stiff k Katı / Ven/	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40% PI :>40%	Yüksek / High Plastic	



Test Pit-33

fuc				-	/ Expert T			İş	veren / Cli	ent		Proje /Proje	ct	
		SIAL		VİRLİK VE	YERBİLİM E MÜHEN . ŞTİ.			K	oza Maden	cilik				
						ARAŞ	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG				
ukur	No / 1	Γest Pit	No				TP-34			Sayfa No / Page No			1	
ütea	ahhit / (	Contrac	tor							YAS Derinliği / Grour	nd water level	(m)		
ukur	Yeri /	Pit Loc	ation							Çukur Derinliği / Dep	th of the Test Pit	(m)	2,15	
arih	/ Date				18.10.20	11				Kot / Elevation		(m)		
azı N	Metodu	ı / Exca	vation N	1ethod	by Excav					Koordinatlar / Coordi	nates	N	4.312.56	37
akin	ie / Ma	chine			Hidromel	220LC						E	678.497	7
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample		Deney / I Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlam	ası / Description	of the Soil		
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)									
3	S-1		D				TS							
<sub>8</sub> .çıkla	ıma / E	Explana	tion:						Üst Görün	üm / Plan View:	Azimut / Az	zimuth:		_
					ı			7014	NI DADA	D HETRELERÍ / SOIL	C (m)	В		(m)
KISA	ALTMA	ALAR / /	ABBREV	/IATION	İNCE	DANELİ / F	INE GRA			LÍ / COARSE GRAINS		STISITE / PLA	ASTICITY	_
P= C	ep Pen t Penet	netromet	Bulk Sam re Ölçüm Enginee	ıü /	N: N: N: N:	0 - 2 Ç.Y 3 - 4 Yun 5 - 8 Orta 9 - 15 Katı	umuşak/Ver nuşak / Soft a Katı / Medi ı / Stiff	y Soft ium	N: 0 - 4 N: 5 - 10 N: 11 - 30 N: 31 - 50 N: > 50	Çok Gevşek/Very Loose Gevşek / Loose Orta Sıkı / Medium Sıkı / Dense Çok Sıkı / Very Dense	PI: 1 - 5% PI:5 - 10% PI:10 - 20% PI:20 - 40% PI:>40%	Çok Az / Trace Az / Low Plastic Orta / Medium I Yüksek / High F Çok Yüksek / V	of Plasticity  Plastic  Plastic	



Test Pit-34

		-	Uzm	an Ekip /	Expert T	eam		İş	veren / Cli	ent		Proje /Projec	t
Ţugi		SIAL		RO SİAL` VİRLİK VE LTD.	MÜHEN			K	oza Maden	cilik			
						ARAŞ	TIRMA	ÇUKU	RU LOGI	U / TEST PIT LOG			
Çukur	No /	Test Pit	No				TP-35			Sayfa No / Page No			1
Mütea	ahhit /	Contrac	ctor							YAS Derinliği / Groun	d water level	(m)	
Çukur	Yeri /	/ Pit Loc	ation							Çukur Derinliği / Dept	h of the Test Pit	(m)	4,50
Tarih					19.10.20					Kot / Elevation		(m)	
		u / Exca	vation N	1ethod	by Excav					Koordinatlar / Coordir	nates	N	4.313.450
Makin	e / Ma	achine			Hidromel	( 220LC		ı				Е	677.373
Derinlik / Depth	Numune No / Sample No	Numune Derinliği / Depth of Sample	Numune Türü / Type of Sample	Yerinde Insitu	Test	Zemin Profili / Soil Profile	Zemin Cinsi / Soil Class			Zemin Tanımlama	ası / Description o	of the Soil	
(m)		(m)		Cinsi / Type	Sonuç / Result (kg/cm²)	# <del>****</del>							
-						د بلد بلد بلد بلد	TS						
- - - 1		1,00											
-	S-1		D				СН						
- 2 - -		2,50											
- 3 - - - - 4	S-2	3,00 4,50	D				СН						
- - 5		4,00											
-													
- 6 Acıkla	ma/F	Explana	tion <sup>.</sup>						Üst Görün	üm / Plan View:	Azimut / Azi	muth:	
r-yiNid	а/ Б	-Apialid	uoil.						ost Gorull	,	A C (m)	В	(m)
KIC	Δ1 T.M./	ΔΙΔΡ / :	ARRDE	/IATION				ZEMİ		METRELERİ / SOIL		8	
NIS/	<b>1            </b>	1LAK / /	-DBKEV	MATION		DANELİ / F		INS	İRİ DANEI	Lİ / COARSE GRAINS	PLAS	TISITE / PLA	
D = To	orba Nu	umune / I	Bulk Sam	ple	N : N :		umuşak/Ver nuşak / Soft	y Soft	N : 0 - 4 N :5 - 10		PI : 1 - 5% PI :5 - 10%	Çok Az / Trace of Az / Low Plastic	of Plasticity
		netromet	re Ölçüm	ıü /	N:		ıuşak / Soit a Katı / Medi	um	N :5 - 10 N :11 - 30		PI :5 - 10% PI :10 - 20%	Orta / Medium Pl	lastic
Pocke		trometer	Enginee	r	N:		/ Stiff	Stiff	N :31 - 50 N :> 50	Siki / Dense	PI :20 - 40%	Yüksek / High Pla	



Test Pit-35

## APPENDIX-2 LABORATORY RESULTS

Yapılan deneylerde TS 1900-1 ve TS 1900-2 standartları kullanılmıştır Laboratuvarımız örnek alımı ve taşınmasından kaynaklanabilecek hatalardan sorumlu değildir. Bu deney sonuçları yalnızca test edilen örnek için geçerlidir Bu deney sonuç tablosu Laboratuvarın izni olmadan kısmen kopyalanamaz, değiştirilemez.

EFOL JEOTEKNÍK HÍZMETLER TÍCARET LÍMÍTED ŞTÍ. Özanadou Sanayi Stresi 677. Sok No.36 OstiniANKARA. Tet (0312) 394 03 61

<b>DENEY!</b>	'APTIRA	DENEYİ YAPTIRAN KURULUŞ							_	YİBF NO:	ö												TARİH	I				
Sayfa:	1/1	Ek:		sayfa deney sonucu	ney sa	nonuc			B	AKA	NE I	(RA	BAKANLIK RAPOR NO:										DEN	DENEY TARİHİ:	રામાં:			
Proje Adı:														핌	NEY S	NO	UÇLA	R Ö	DENEY SONUÇLARI ÖZET TABLOSU	BLOS	<u>ب</u>							
			В	Elek A	ınalizi	Elek Analizi / Sieve Analysis	'e Ana	llysis		Att	Atterberg Limit/At.Limits	g nits	sosn	Birim / I We	Birim Ağırlık / Unit Weiaht	Ω̈́ F	Üç eksenli / Triaxial Test		Konsolidas./ Consolidation	<b>—</b>		ΞŢ	Hidrometri / Hydrometer	i/ er		Prok	Proktor / Proctor Test	tor
I	HIMMETDEDE	EDE	bodjeM \ tot	<del>1/#</del> -	0l#-	0 <del>7#-</del>	-#500	nO	TD Utying	tma / Drying	LL PL	□.	Birleştirilmiş Zemin Sınıfı / Unified Soil Classification System	Su Oranı \ Water Cont. Doğal B.A. \ Natural U.W.	.A.B אורע B.W.U (	iqiT (ənə	kg/cm²	Derece Doğal Boşluk	Oranı / Natural Void Ratio Sonraki Boşluk	Oranı / Void Ratio After Test Özgül Ağırlı	Spesific Gra	tot / Method Vğırlık kaybı /	Neight Loss	200,0-	PERMEABİL Permeabili	bott / Method	Maximum Kuru Bir.Ağ. / Max. Dry Unit Weight. Optimum Su	lçeriği / Optim. Water Content
Kuyu No /BH No	Örnek / Sample No	Derinlik / Depth	эМ	%	%	%	%		ZI	nun %	% %	%	sosn	ub %	gr/cm³	3	O	Ø	- - -	e <sub>f</sub> gr/	gr/cm³	1 %	%	%	cm/sn(*10 <sup>-7</sup> )		gr/cm³	%
TP-1	S-1	0,45-1,80	guiv	89	2	22	31		Saiv	gniy ജ	4 25	6	SM	13									30	13				
TP-2	S-1	0,50-1,40	vəiS	92	88	75	52		ى u		40 29	1	ML	16									52	4				
TP-3	S-1	0,50-1,50	ίθW	92	85	75	65		3/4/		45 28	17	ML	16									65	26				
TP-3	S-2	1,50-2,40	/ əu	62	26	46	36		, ,		42 27	15	ML	14									36	10				
TP-4	S-1	0,60-3,00	uəl∃	63	49	28	17			12 12	7 22	5	SM	7									17	4				
TP-5	S-1	0,40-1,55	nalı	70	26	37	27		· A 6	nv ə ⊝	1 24	7	SM	2									27	7				
TP-6	S-1	2,20-3,40	ıkar	66	86	94	06		P.V.TH		95 49	46	MH	45						2	2,54		6	40	1,08		1,040	52,7
TP-7	S-1	0,80-3,00	(	94	83	09	45		-3		47 30	17	SM	24									45	8				
TP-8	S-1	0,80-2,00		48	36	22	10	108 2	2,4	3	30 27	ო	GW-GM	7									10	က				
TP-9	S-1	0,40-1,40		66	97	83	09			ര്	36 25	11	ML	19									9	59				
TP-11	S-1	0,35-1,80		98	96	91	83			7	70 34	. 36	ᆼ	32						2	2,46		83	42	1,87		1,218	39,3
TP-11	S-2	1,30-4,00		66	86	92	89			ŏ	82 46	36	HW	33						2	2,44		88	40	1,52		1,137 4	42,7
TP-12	S-1	0,80-2,25		66	92	70	44			4	45 30	15	SM	25									4	10				
TP-13	S-1	0,60-2,65		82	99	42	29		$\dashv$	4	47 28	19	SM	25									29	6				
TP-14	S-1	0,60-1,20		88	81	61	44			ਲ	38 30	8	SM	20									4	6				
		Laboratuv	/arın	nız B	ayın	dirli	c ve i	skan	ו Bal	kan	Ē	Үар	Laboratuvarımız Bayındırlık ve İskan Bakanlığı Yapı İşleri Genel Müdürlüğü tarafından verilen 11 numaralı	el Müdü	ırlüğü	tara	fında	n ver	ilen 11	numa	<u>ra</u>							
		'Laboratuvar İzin Belgesi'ne sahiptir.	vari	izin E	3elg(	esi'n(	e sah	iptir	. •																			
	_		ľ	ŀ	í	300	].		3	,	[	ŀ												_				

EFOL JEOTEKNÍK HÍZMETLER
TÍCARET LÍMÍTED ŞTÍ.
Özanadolu Senayi Stesi 677. Sol. No.36 Ostm/ANKARA.
Tet (0312) 394 03 61

<b>DENEY!</b>	(APTIRA	DENEYI YAPTIRAN KURULUS	ļ						F	YİBF NO:	ö				-							Ė	TARİH					Γ
Sayfa:	1/1	Ek :		sayfa deney sonucu	ey so	nonu			B	BAKANLIK		RAPC	RAPOR NO:		H								<b>JENEY</b>	DENEY TARİHİ	ij:			
Proje Adı:														DE	>	ONO	ÇLA	SONUÇLARI ÖZET TABLOSU	T TAB	LOSU								
			Ш	lek Ar	اalizi /	Elek Analizi / Sieve Analysis	, Anal	ysis		Atterberg Limit/At.Lim	Atterberg nit/At.Limit	its	nscs	Birim Ağırlık / Unit Weight		Üç eksenli / Triaxial Test	enli / Test	Kons	Konsolidas./ Consolidation	K /		Hidro	Hidrometri / Hydrometer			Proktor /	Proktor / Proctor Test	r Test
I	HİMMETDEDE	)EDE	bort Method	₽#-	0l#-	0 <del>1/#-</del>	002#-	Cu	Cr tma / Drying		PL	PI CI	Birleştirilmiş Zemin Sınıfı / E Unified Soil Classification System	Su Oranı / Water Cont. Doğal B.A. / Natural U.W.	Dry U:W: eney Tipi	kg/cm²	Derece	Doğal Boşluk Oranı / Natural Void Ratio	Sonraki Boşluk Oranı / Void Ratio After Test	IlyigA lügzÖ rsıð offisəq2	tot / Method	ığıtlık kaybı / Veight Loss	940'0-	200,0-	PERMEABILI Permeabili	tot / Method Maximum Kuru	Bir.Ağ. / Max. Dry Unit Weight. Optimum Su	İçeriği / Optim. Water Content
Kuyu No /BH No	Örnek / Sample No	Derinlik / Depth	9M	%	%	%	%		Kıırı	%	%	%	nscs	l O		l		ข็	ά	gr/cm³	϶M		%	%	cm/sn(*10 <sup>-7</sup> )	<u> </u>	n³	%
TP-15	S-1	0,60-2,50		66	95	, 92	49	H	nniv	4	27	13	SM	25									49	10				
TP-15	S-2	2,50-4,20		86	97	92	84		ս Ու	53	29	24	СН	25									84	48				
TP-21	S-1	0,97-1,60	təW	81	11	20	27		i9wC	4	22	19	SM	23									27	7				
TP-22	S-1	1,50-3,00		92	92	. 48	73		, / et	52	32	20	MH	26									73	27				
TP-22	S-2	3,00-3,20		86	96	91	83		սիլոր	51	35	16	M	26									83	33				
TP-23	S-1	3,30-3,40		100	100	86	92		€ Kı ∏	26	28	28	CH	09									95	89				
TP-24	S-1	1,00-1,90		92	88	84	80		hvüJ	28	26	32	MH	20									80	37				
TP-25	S-1	0,30-0,80		89	83	70	58			45	25	20	ر ا	20									58	32				
TP-25	S-1	0,80-3,00		92	91	86	81			54	31	23	MH	25									80	35				
TP-26	S-1	0,30-1,50		96	91	80	65			33	22	11	C	21									65	40				
TP-26	S-2	0,50-1,40	ν-	100	100	. 63	27			28	38	20	MH	36									92	37				
TP-31	S-1	0,40-4,40		66	97	83	65			45	17	28	ر ا	19									65	35				
TP-34	S-1	0,40-2,15		85	82	99	52			31	78	က	ML	8									52	17				
TP-35	S-1	1,00-2,50	Υ-	100	86	93	88			63	28	35	CH	21						2,61			88	74	9,0	1,3	1,365	30,41
TP-35	S-2	3,00-4,50		86	92	90	84			64	30	34	CH	25	-					2,61			84	53	1,14	-	359 3	33,7
		Laboratuvarımız Bayındırlık ve İskan Bakanlığı Y	arım Azri	ıız B.	ayınc	il k	ve k	skan	Ba	kanl			apı İşleri Genel Müdürlüğü tarafından verilen 11 numaralı	ı Müdürli	iğü ta	rafin	dan 🕯	verilen	11 nur	naralı								
		Yanılan de	AVen	a draw	15. T	900-1	\ \ \ \	7	900-	200	Puc	r In	Vapilan denevlerde TS 1900-1 ve TS 1900-2 standartları kı illanılmıstır	HI									T					
		Laboratuva	arımı.	zörn	ek al	ž E	e taş	inme	SINC	an l	cayna	aklan	Laboratuvarımız örnek alımı ve taşınmasından kaynaklanabilecek hatalardan sorumlu değildir.	atalardan	sorum	ilu de	ğildir											
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Appendix C:	Water Balance Calculations	S



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A water balance model was developed to evaluate the solution pond capacity, make-up water, and water management requirements for average monthly conditions using an Excel™ spreadsheet developed specifically for the Himmetdede On-Off Heap Leach Pad (OHLP) Project.

Deterministic analyses were performed for the 8-year mine life, corresponding to 190 cycles, with each cycle corresponding to a 14-day cell size for steady state conditions (not including ramp up or shut down). The Excel spreadsheet was used to simulate the pad loading (including ore loading, lined pad and pond areas), track inflows (such as wet-up of ore, rainfall, solution application, draindown, etc.) and outflows (such as pad and pond evaporation, etc.). A detailed discussion for each of the items considered in the water balance, along with a description of the calculation, is presented below.

The Solution Pond system was developed utilizing the following mass balance systems as described below and presented in Appendix C:

- Mass Balance 1 (OHLP Active Leaching Cells): Ore enters into the system from the mine where it
  is agglomerated and stacked on to the OHLP. Barren Solution enters Mass Balance 1 from the
  Process Plant, and is either used to wet the ore in the Agglomeration drum or is used to leach the
  ore. Pregnant solution is collected and reports to the Pregnant Pond, where gold is recovered
  and Barren Solution is re-introduced into the system. Mass Balance 1 is then used to size the
  Pregnant Pond;
- Mass Balance 2 (OHLP Rinsing Cells): At the end of leaching, ore from mass Balance 1 is rinsed
  with freshwater for 14 days to remove process solution. The Rinse Solution then reports to a
  Rinse Pond, where it is pumped to the Process Plant and introduced into Mass Balance 1 as
  makeup water. Mass Balance 2 is then used to size the Rinse Pond, and any excess Rinse
  solution that the Process Plant cannot accept must be treated and discharged; and
- The Storm Pond sizing is independent of the Mass Balances, and based on the design storm event depth, multiplied by the total OHLP and Pond areas.

The results of the water balance for the active leaching cells (Mass Balance 1) indicate that the system is a net user, or requires makeup water addition. However, the rinse cells (Mass Balance 2) are a net water producer, or generates excess water. SRK investigated combining the two systems, performing a combined mass balance between excess water created annually by the rinsing system against the amount of make-up water by altering the application rate of the freshwater rinse solution. The effect of this overall water balance calculated a maximum rinse application rate of 1.50 l/hr/m2 in order to achieve a net balance on an annual basis between Mass Balance 1 and Mass Balance 2. In other words, if the rinse application rate exceeds 1.50 l/hr/m2, then water treatment of the rinse solution will be required. If the application rate is less than this amount then make-up water will be required for the process plant.

The water balance was also used as a basis to size three external solution ponds (Rinse, Pregnant, and Stormwater Ponds), which are located downstream of the OHLP facility. The Pond capacity for each of the ponds was then calculated in 14-day increments considering the following variables:

- Maximum operating volume. The maximum operating depth was assumed to be approximately 5m for the Pregnant and Storm Ponds;
- Draindown. This represents an estimate of the maximum period of time that pond pumps may be without power, pumps be maintained or replacement pumps installed, and unable to recirculate solution;
- Variations in precipitation and evaporation. The changes in volume due to seasonal changes in precipitation and evaporation were considered;
- Design storm event. The runoff generated from the 100-year, 24-hour storm event was used as the design storm event and was assumed by SRK to be 100mm due to lack of design storm precipitation information for this part of Turkey; and



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 Design storm event. The runoff generated from the 100-year, 24-hour storm event was used as the design storm event and was assumed by SRK to be 100mm due to lack of design storm precipitation information for this part of Turkey; and

Freeboard. A minimum freeboard of 0.5m in the ponds.

In summary, the ponds have been sized considering they are at the maximum operating capacity, when there is either a loss in power such that the irrigation pumps are down or the pumps are being maintained or replaced and all of the ore is draining down at a rate of 10 L/hr/m² for a period of 6 hours (24 hours at a rate of 1.5 L/hr/m² for the rinse cells, and the 100-year, 24-hour storm event occurs at the same time.

The water balance model was used to calculate the following pond storage volumes:

- Pregnant Pond. The volume was determined considering each of the inflows and outflows and a six hour draindown period;
- Stormwater Pond. The volume was calculated by multiplying the 100-yr, 24-hr storm event rainfall depth by the total lined pad and design pond area. This pond will be maintained dry for most of the operational life except when there are large storms and/or excess flows are directed to it via spillways from the other ponds; and
- Rinse Pond. This volume was determined, considering each of the inflows and outflows and a 24 hour draindown period. The Rinse Pond volume was determined by assuming that the pond would contain all the excess flow from periods of high precipitation through the evaporation cycle where this solution would have an opportunity to evaporate naturally, creating a yearly net inflow water balance of zero.

The Rinse application rate was optimized at 1.5 L/hr/m² so that the Rinse Pond volume could be calculated for an annual net inflow water balance of zero. This application rate may need to be increased depending upon geochemistry testing results during the next design phase of the project. The calculated storage volume below freeboard levels (consisting of the maximum operating volume and draindown) along with design storage volumes required for each of the ponds are summarized below. The design storage volume also consists of the additional storage volume from the sloped bottom of each pond.

**Table 1: OHLP Pond Sizing** 

Pond	Maximum Operating Volume including Precipitation Variation (m³)	Irrigation Draindown* (m³)	Calculated Storage Volume (m³)	Design Storage Volume (m³)
Pregnant	0	7,511	7,511	8,675
Rinse	5,407	225	5,632	38,000**
Stormwater	0	0	27,000	27,000***

<sup>\*6</sup> hours for pregnant solution draindown from 5 active leach cells and 24 hours for draindown from one active rinse cell.

The design storage volume is typically slightly greater than the calculated storage volume required as the pond dimensions were usually rounded up to the nearest 0.5m increment, while maintaining an overall constant pond width.

<sup>\*\*</sup>Design storage volume calculations based on the rinse pond holding the yearly rinse water balance without treatment or make-up requirements.

<sup>\*\*\*</sup>Design storage volume calculations assumed no pond volume except for storm water overflow from pregnant and rinse ponds.



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A description of the line item components of the water balance model is presented below.

- Column A 14-Day Cycle Indicates the number of 4-Day cycles.
- Column B Start Date Indicates the predicted start date of each cycle in terms of month and day. This model assumes operations to begin October 1, 2012.
- Column C End Date Indicates the end date of a 14-day cycle.
- Column D Climatic Data, Precipitation per Cycle (mm) Presents the time weighted value of average monthly precipitation data over a 14-day cycle in millimeters. Monthly precipitation data per project design criteria (Appendix A).
- Column E Climatic Data, Evaporation per Cycle (mm) Presents the time weighted value of average monthly evaporation data over a 14-day cycle in millimeters. Monthly evaporation data per project design criteria (Appendix A).
- Column F Operation Data, Ore Production (tonnes) Calculates the weight of ore produced over a 14-day cycle at 11,000 tonnes per day.
- Column G Operation Data, Cumulative Ore Production (tonnes) Calculates the cumulative weight of ore produced over the life mining operations.
- Column H Variable, Loaded Ore Volume (m3) Calculates the volume of ore production over a 14-day cycle by dividing Ore Production (Column F) by the density of the ore (Input Sheet reported in units of tonnes per m3).
- Column I Variable, Stacking Cell Area (m2) Calculates the area of the stacked ore on the pad by dividing the Loaded Ore Volume (Column H) by the lift height of the cell (Input Sheet – reported in units of m).
- Column J Variable, Irrigation Cell Areas (m2) Calculates the area under irrigation on the pad by multiplying the Stacking Cell Area (Column I) by the number of cells under leach (Maximum of 5).
- Column K Variable, Downdrain Cell Areas (m2) Calculates the area of cells under post leaching draindown. Fixed number beginning in cycle 7.
- Column I Variable, Rinse Cell Area (m2) Calculates the area of cells under rinse. Fixed number beginning in cycle 8.
- Column M Variable, Unloading Cell Area (m2) Calculates the area of cells being unloaded after the rinse cycle is completed. Fixed number beginning in cycle 9.
- Column N Variable, Contingency Cell Area (m2) Calculates the area of cells being used for contingency. Fixed number beginning in cycle 9.
- Column 0 Mass Balance 1, Inflows, Irrigation from Process Plant (m3/hr) Calculates the flow rate of process solution entering mass balance 1 from the process plant. Process solution



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entering into mass balance 1 goes to the agglomeration process and the leach pad for leaching at a rate of 10 l/m2/hr applied over the area of a maximum of five active leaching cells. Number becomes fixed once process reaches steady state. Column O links directly to and equals Column A7.

- Column P Mass Balance 1, Inflows, Solution Application (Volume) from Process Plant (m3) –
  Calculates the flow of process solution entering mass balance 1 from the process plant in terms
  of volume per cycle. Calculated by multiplying the Irrigation from the Process Plant (Column O)
  by the number of days per cycle (14) and the number of hours per day (24).
- Column Q Mass Balance 1, Inflows, Initial Ore Water Inflow (m3) Calculates the amount of
  water incoming into mass balance 1 contained in the ore being delivered from the mine.
  Calculated by multiplying the Ore Production (Column F) by the Initial Water Content (Input Sheet
   reported as a percentage).
- Column R Mass Balance 1, Inflows, Precipitation on Irrigation and Draindown Cell Areas (m3) –
  Calculates the amount of precipitation falling onto the areas of the irrigation and draindown cells.
  Calculated by adding the Irrigation Cell Areas (Column J) and the Downdrain Cell Areas (Column K) together and multiplying the sum by the Precipitation per Cycle (Column D).
- Column S Mass Balance 1, Inflows, Precipitation on Stacking Cell Area (m3) Calculates the
  amount of precipitation falling onto the Stacking Cell. Calculated by multiplying the Stacking Cell
  Area (Column I) by the Precipitation per Cycle (Column D), dividing by 1000 to convert Column D
  mm to m, and multiplying by the Inactive Runoff Factor (Input Sheet).
- Column T Mass Balance 1, Inflows, Total Precipitation (m3) Calculates the total precipitation for mass balance one. Calculated by summing Precipitation on Irrigation & Draindown Cell Areas (Column R) and Precipitation on Stacking Cell Area (Column S).
- Column U Mass Balance 1, Inflows, Total Inflow (m3) Calculates the total inflow into mass balance 1. Calculated by summing Solution Application (Volume) from Process Plant (m3) (Column P), Initial Ore Water Inflow (Column Q), Precipitation on Irrigation & Draindown Cell Areas (Column R), and Precipitation on Stacking Cell Area (Column S).
- Column V Mass Balance 1, Outflows, Evaporation from Irrigation & Draindown Cell Area (m3) –
  Calculates the evaporation from the irrigation and draindown cells. Calculated by summing
  Irrigation Cell Areas (Column J) and Downdrain Cell Areas (Column K), multiplying the sum by
  Evaporation per Cycle (Column E), dividing by 1000 to convert mm to m, and multiplying by the
  Solution Application Evaporation Factor (Input Sheet reported as a fraction of applied solution
  that will evaporate from the ore).
- Column W Mass Balance 1, Outflows, Evaporation from Stacking Cell Area (m3) Calculates
  the evaporation from the stacking cell area. Calculated by multiplying the Stacking Cell Area
  (Column I) by the Evaporation per Cycle (Column E), dividing by 1000 to convert mm to m, and
  multiplying by the Solution Application Evaporation Factor (Input Sheet reported as a fraction of
  applied solution that will evaporate from the ore).
- Column X Mass Balance 1, Outflows, Total Evaporation (m3) Calculates the total evaporation for mass balance 1. Calculated by summing the Evaporation from Irrigation & Draindown Cell Area (Column V) and Evaporation from Stacking Cell Areas (Column W).



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- Column Y Mass Balance 1, Outflows, Ore Water Outflow (m3) Calculates the amount of water contained in the ore that transfers from the draindown cell in mass balance 1 to the rinse cell in mass balance 2. Calculated by multiplying the Ore Production (Column F) by the Residual Moisture Content (Input Sheet reported as a percentage). Reaches steady state during cycle 7.
- Column Z Mass Balance 1, Outflows, Total Outflow (m3) Calculates the total outflow from mass balance 1. Calculated by summing Total Evaporation (Column X) and Ore Water Outflow (Column Y).
- Column AA Mass Balance 1, Outflows, Leached Solution to Pregnant Pond (m3) Calculates
  the amount of remaining water from mass balance 1 available to report to the process plant.
  Calculated by subtracting Total Outflows (Column Z) from Total Inflows (Column U).
- Column AB Mass Balance 2, Inflows, Freshwater Flow Rate Required (m3/hr) Calculates the flow rate required to rinse the spent ore. Calculated by multiplying the Rinse Application Rate (Input Sheet – reported as I/hr/m2) by the Rinse Cell Area (Column L). Reaches steady state during cycle 8, the first rinse cell cycle.
- Column AC Mass Balance 2, Inflows, Freshwater (Volume) Required (m3) Calculates the freshwater required to rinse the spent ore. Calculated by multiplying the Freshwater Flow Rate Required (Column AB) by the number of days per cycle (14) and the number of hours per day (24). Reaches steady state during cycle 8.
- Column AD Mass Balance 2, Inflows, Ore Water Inflow (m3) Calculates the amount of water in the ore coming in to mass balance 2. Calculated by multiplying Ore Production (Column F) by the Residual Moisture Content (Input Sheet reported as a percentage).
- Column AE Mass Balance 2, Inflows, Precipitation on Rinse & Unloading Cell Areas (m3) –
  Calculates the precipitation on the rinse and unloading cell areas. Calculated by summing the
  Rinse Cell Area (Column L) and the Unloading Cell Area (Column M), multiplying by the
  Precipitation per Cycle (Column D), dividing by 1000 to convert mm to m, and multiplying by the
  Inactive Runoff Factor (Input Sheet).
- Column AF Mass Balance 2, Inflows, Precipitation on Contingency Area (m3) Calculates the precipitation on the contingency cell area. Calculated by multiplying the Contingency Cell Area (Column N) by the Precipitation per Cycle (Column D) and dividing by 1000 to convert mm to m.
- Column AG Mass Balance 2, Inflows, Total Precipitation (m3) Calculates the total precipitation on mass balance 2. Calculated by summing Precipitation on Rinse & Unloading Areas (Column AE) and Precipitation on Contingency Area (Column AF).
- Column AH Mass Balance 2, Inflows, Total Inflows (m3) Calculates the total precipitation into mass balance 2. Calculated by summing Freshwater (Volume) Required (Column AC), Ore Water Inflow (Column AD), Precipitation on Rinse & Unloading Cell Areas (Column AE), and Precipitation on Contingency Area (Column AF).
- Column AI Mass Balance 2, Outflows, Evaporation on Rinse & Unloading Cell Areas (m3) Calculates the evaporation on the rinse and unloading cells in mass balance 2. Calculated by summing Rinse Cell Area (Column L) and the Unloading Cell Area (Column M), multiplying by the Evaporation per Cycle (Column E), dividing by 1000 to convert mm to m, and multiplying by the



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Solution Application Evaporation factor (Input Sheet – reported as a the fraction of applied solution that will evaporate from the ore).

- Column AJ Mass Balance 2, Outflows, Ore Water Outflow (m3) Calculates the amount of
  water leaving mass balance 2 in the ore and going to the spent ore facility. Calculated by
  multiplying the Ore Production (Column F) by the Residual Moisture Content (Input Sheet –
  reported as a percentage). Reaches steady state during cycle 9.
- Column AK Mass Balance 2, Outflows, Total Outflow (m3) Calculates the total outflow of water from mass balance 2. Calculated by summing Evaporation on Rinse & Unloading Cell Areas (Column AI) and Ore Water Outflow (Column AJ).
- Column AL Mass Balance 2, Outflows, Rinse Solution to Rinse Pond (m3) Calculates the solution that reports to the rinse pond. Calculated by subtracting Total Outflow (Column AK) from Total Inflow (AH).
- Column AM Pregnant Pond, Inflows, Daily Solution Flowrate from Pad (m3/hr) Calculates the flow rate of solution from mass balance 1 to the pregnant pond. Calculated by dividing the volume of Leached Solution to Pregnant Pond (Column AA) by the number of days per cycle (14) and the number of hours per day (24).
- Column AN Pregnant Pond, Inflows, Pond Precipitation (m3/hr) Calculates the flow rate of precipitation into the pregnant pond. Calculated by multiplying Precipitation per Cycle (Column D) by the Pregnant Pond Top Area (Input Sheet reported as m2), dividing by 1000 to convert mm to m, dividing by number of days per cycle (14), and dividing by number of hours per day (24).
- Column AO Pregnant Pond, Inflows, Total Inflow (m3/hr) Calculates the total inflow into the pregnant pond. Calculated by summing Daily Solution Flow Rate from Pad (Column AM) and Pond Precipitation (Column AN).
- Column AP Pregnant Pond, Outflows, Pond Evaporation (m3/hr) Calculates the flow rate of evaporation from the pregnant pond. Calculated by multiplying Evaporation per Cycle (Column E) by the Pregnant Pond Top Area (Input Sheet reported as m2), dividing by 1000 to convert mm to m, dividing by number of days per cycle (14), and dividing by number of hours per day (24).
- Column AQ Pregnant Pond, Outflows, Total Outflow (m3/hr) Calculates the total outflow from evaporation from the pregnant pond. Calculated as the Pond Evaporation (Column AP)
- Column AR Pregnant Pond, Operating Pond Volume for 2-m head (m3) Calculates the
  volume in the pregnant pond necessary to maintain 2m of head to prevent pump cavitation.
  Calculated by the larger of the two numbers Pregnant Pond Volume 2m Head (Input Sheet –
  reported as m3) and Daily Pond Storage (Column AS).
- Column AS Pregnant Pond, Daily Pond Storage (m3) Calculates the necessary pregnant pond storage to contain the required draindown volume from mass balance 1. Calculated by multiplying the Pregnant Pond Draindown Capacity (Input Sheet – reported as hours) by the Net Inflow to Process Plant (Column AT).



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- Column AT Pregnant Pond, Net Inflow to Process Plant (m3/hr) Calculates the flow rate of solution into the process plant from the pregnant pond. Calculated by subtracting the Total Outflow (Column AQ) from the Total Inflow (Column AQ).
- Column AU Process Plant, Inflows, Pregnant Pond (m3/hr) Calculates the flow rate into the process plant from the pregnant pond. Calculated as the sum of Net Inflow to Process Plant (Column AT).
- Column AV Process Plant, Inflows, Rinse Pond (m3/hr) Calculates the inflow rate that the process plant can accept from the rinse pond to maintain a zero net inflow in the process plant. Calculated as the sum of To Process Plant (Column BF).
- Column AW Process Plant, Inflows, Total Inflow (m3/hr) Calculates the total inflow rate to the process plant. Calculated by summing Pregnant Pond (Column AU) and Rinse Pond (Column AV).
- Column AX Process Plant, Outflows, To Agglomerator (m3/hr) Calculates the flow rate of solution necessary to send to the agglomerator to increase the incoming ore from the mine from initial water content to the reported agglomerator water content. Calculated by subtracting the Initial Water Content (Input Sheet reported as a percentage) from the Agglomeration with Freshwater (Input Sheet reported as a percentage), multiplying by the Ore Production (Column F), dividing by the number of days per cycle (14), and dividing by the number of hours per day (24). Reaches steady state during cycle 1.
- Column AY Process Plant, Outflows, To Leach Cells (m3/hr) Calculates the amount of solution flow rate required for application to the leaching cells. Calculated by multiplying the Irrigation Cell Areas (Column J) by the Solution Application Rate (Input Sheet reported as I/hr/m2) and divided by 1000 to convert I to m3. Reaches steady state during cycle 6.
- Column AZ Process Plant, Outflows, Total Outflow (m3/hr) Calculates the total outflow rate from the process plant to mass balance 1. Calculated by summing To Agglomerator (Column AX) and To Leach Cells (Column AY).
- Column BA Process Plant, Net Inflow (m3/hr) Verifies the net inflow rate to the process plant assuming a net inflow of zero. Calculated by subtracting Total Outflow (Column AZ) from Total Inflow (Column AW).
- Column BB Rinse Pond, Inflows, Rinse Solution Inflow (m3/hr) Calculates the flow rate into the rinse pond from mass balance 2. Calculated by multiplying Rinse Solution to Rinse Pond (Column AL), divided by number of days per cycle (14), and divided by number of hours per day (24).
- Column BC Rinse Pond, Inflows, Pond Precipitation (m3/hr) Calculates the flow rate of precipitation into the rinse pond. Calculated by multiplying the Precipitation per Cycle (Column D) by the Rinse Pond (Input Sheet reported as m2), divided by 1000 to convert mm to m, divided by number of days per cycle (14), and divided by number of hours per day (24).
- Column BD Rinse Pond, Inflows, Total inflows (m3/hr) Calculates the total inflow into the rinse pond. Calculated by summing the Rinse Solution Inflow (Column BB) and Pond Precipitation (Column BC).



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- Column BE Rinse Pond, Outflows, Pond Evaporation (m3/hr) Calculates the evaporation flow rate from the rinse pond. Calculated by multiplying the Evaporation per Cycle (Column E) by the Rinse Pond Area(Input Sheet reported as m2), divided by 1000 to convert mm to m, divided by number of days per cycle (14), and number of hours per day (24).
- Column BF Rinse Pond, Outflows, To Process Plant (m3/hr) Calculates the flow rate of solution that the process plant can accept from the rinse pond assuming a net flow rate in the process plant. Calculated by subtracting Pregnant Pond (Column AU) from Total Outflow (Column AZ).
- Column BG Rinse Pond, Outflows, Total Outflow (m3/hr) Calculates the total outflow rate from the rinse pond. Calculated by summing Pond Evaporation (Column BE) and To Process Plant (BF).
- Column BH Rinse Pond, Net Inflow (m3/hr) Calculates the total inflow rate of the rinse pond. This inflow rate indicates the amount of water that must be added for makeup purposes or treatment if excess. Calculated by subtracting Total Outflow (Column BG) from Total Inflow (Column BD).
- Column BI Rinse Pond, Operating Pond Volume for 2m Head (m3) Calculates the minimum volume required to maintain 2m of pond depth to avoid cavitation. Calculated by taking either the Rinse Pond Volume 2m Head (Input Sheet reported as m3) or Daily Pond Storage (Column BJ) amount, whichever is larger.
- Column BJ Rinse Pond, Daily Pond Storage (m3) Calculates the amount of required storage to hold the drain down event based on a specified hourly input. Calculated by multiplying the Net Inflow (Column BH) by the Rinse Draindown Pond Storage Capacity (Input Sheet reported in hours).
- Column BK Rinse Pond, Make-Up Volume (m3/hr) Calculates the amount of makeup water addition that must occur to keep the system at equilibrium based on the water balance. Calculated by subtracting Total Inflow (Column BD) from To Process Plant (Column BF) if To Process Plant amount is larger than Total Inflow.
- Column BL Rinse Pond, Excess Pond Volume (m3/hr) Calculates the amount of water treatment that must occur to keep the system at equilibrium based on the water balance. Calculated by summing Net Inflow (Column BH) if Total Outflow (Column BG) is greater than zero.



Table 1: Input Variables and Constants

Calutian Annication	Overstite	***Madel accuracy all analysis and a sector according to the Company Device Dev
Solution Application	Quantity Units	***Model assumes all agglomeration water comes from Barren Pond
Solution Application Rate:	1.505 l/hr/m <sup>2</sup>	4 F4 Maylar 2 in Equality of Rote and annuary 27 000 and around
Rinse Application Rate: Cell Size:		1.54 l/hr/m2 is Equalized Rate w/ approx 37,000 m3 wash pond
	14 days	1 cell
Stacking: Irrigation	14 days 70 days	5 cells
Downdrain Option A:	14 days	1 cell Freshwater Applied in Mass Balance 2
Downdrain Option B	0 Days	0 cell Freshwater Applied in Mass Balance 1
Rinse:	14 days	1 cell
Unloading:	14 days	1 cell
Contingency:	14 days	1 cell
Subtotal:	140 days	10 cells
Cycle Rate:	14 days/cycle	10 00113
Start Date:	10/1/2012 mm/dd/yyyy	
Solution Application Evaporation Factor:	25.0%	
Pregant Pond Draindown Capacity:	6 hours	
Barren Pond Draindown Capacity:	0 hours	
Rinse Downdrain Pond Storage Capacity:	24 hours	
<b>.</b> ,		
Areas	Quantity Units	
Total Lined Area:	250,000 m <sup>2</sup>	
No. of Cells:	10	
Cell Area:	25,000 m <sup>2</sup>	
Active Leach Area:	120,313 m <sup>2</sup>	
Pregnant Pond:	2,500 m <sup>2</sup>	
Barren Pond:	2,500 m <sup>2</sup>	
Rinse Pond:	16,732 m <sup>2</sup>	
Storm Pond:	9,600 m <sup>2</sup>	
Total Lined Pond Area:	31,332 m <sup>2</sup>	
	- 1,0-1	
Inactive Runoff Factor:	1.00	
Pregnant Pond Volume 2m Head:	82.70	
Rinse Pond Volume 2m Head:	26.20	
Barren Pond Volume 2m Head:		
Ore Production	Quantity Units	
Ore Production rate:	11,000 tonnes/day	
Heap Average Stacked Unit Weight:	1.6 tonnes/m <sup>3</sup>	
Design Life:	10.0 years	
Initial Water Content:	3% (by weight)	
Agglomeration with Freshwater:	9% (by weight)	
Leaching Water Content:	18.0% (by weight)	
Residual Moisture Content:	12.0% (by weight)	
Lift height:	4.00 m	
No. of Lifts:	1	



### **Table 2: Climate Data**

**Precipitation and Evaporation** 

	Precipitation	Evaporation
	Average Monthly	Average Monthly
Month	(mm)	(mm)
January	31.7	0
February	33	0
March	41.1	1
April	56.3	58
May	57.4	125
June	36	172
July	13.1	217
August	6.5	202
September	11.4	140
October	33.3	69
November	38.2	2
December	39.4	0
Average Annual	397.4	984.2

## Storm Events

100-yr, 24-hr	100	mm

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39	2 C	36	35	34	33	32	31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	1	10	9	<b>∞</b>	7	6	σı .	4	ι ω	v	_		14-Day Cyc	le	
17-Mar	17-Feb	3-Feb	20-Jan	6-Jan	23-Dec	9-Dec	25-Nov	11-Nov	28-Oct	14-0ct	30-Sep	16-Sep	2-Sep	19-Aug	5-Aug	22-Jul	8-Jul	24-Jun	10-Jun	27-May	13-May	29-Apr	15-Apr	1-Apr	18-Mar	4-Mar	18-Feb	4-Feb	21-Jan	7-Jan	24-Dec	10-Dec	26-Nov	12-Nov	29-Oct	15-Oct	1-0ct		Start Date	•	
30-Mar	16 Mar	16-Feb	2-Feb	19-Jan	5-Jan	22-Dec	8-Dec	24-Nov	10-Nov	27-Oct					18-Aug	4-Aug	21-Jul	7-Jul	23-Jun	9-Jun	26-May	12-May	28-Apr	14-Apr	31-Mar	17-Mar	3-Mar	17-Feb	3-Feb	20-Jan	6-Jan	23-Dec	9-Dec	25-Nov	11-Nov	28-Oct	14-0ct		End Date		
18.56	16.79	16.50	14.63	14.32	16.55	17.79	17.81	17.83	17.03	15.04	14.34	5.32	5.32	3.11	2.94	5.06	5.92	11.36	16.80	20.06	25.92	25.97	26.27	26.27	18.56	18.56	16.94	16.50	14.78	14.32	16.30	17.79	17.81	17.83	17.23	15 04	15.04	(mm)	Precip. per Cycle	Climatic Data	:
0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.8	9.5	31.1	33.6	65.5	65.5	89.2	91.0	95.9	97.8	89.0	80.3	71.7	56.2	52.1	27.1	27.1	0.3	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.3	0.8	7.3	31	31.1	(mm)	Evap. per Cycle	c Data	
154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154.000	154.000	154 000	154,000	(tonnes)	Ore Prod.	Operation Data	·
6,006,000	5,698,000	5,544,000	5,390,000	5,236,000	5,082,000	4,928,000	4,774,000	4,620,000	4,466,000	4,312,000	4,158,000	4,004,000	3,850,000	3,696,000	3,542,000	3,388,000	3,234,000	3,080,000	2,926,000	2,772,000	2,618,000	2,464,000	2,310,000	2,156,000	2,002,000	1,848,000	1,694,000	1,540,000	1,386,000	1,232,000	1,078,000	924,000	770,000	616.000	462.000	308 000	154,000	(tonnes)	Cum. Ore Prod.	on Data	l
96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96,250	96.250	96.250	96 250	96,250	(m³)	Loaded Ore Vol.		
24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24.063	24.063	24 063	24,063	(m²)	Stacking Cell Area		
120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	120,313	96,250	72.188	48.125	24 063	0	(m²)	Irrigation Cell Areas		
24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	0	0	0	0	0	0	(m²)	Downdrain Cell Areas	Variable	
24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	0	0	0	0	0	0	0	(m²)	Rinse Cell Area		
24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	0	0	0	0	0	0	0	0	(m²)	Unloading Cell Area		
24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	24,063	48,125	72,188	96,250	120,313	144.375	168.438	192 500	216,563	(m²)	Contingency Cell Area		
1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	1,231	990	749	509	268	27.5	(m³/hr)	Irrigation from Process Plant		
413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	413,490	332,640	251.790	170.940	90 090	9,240	(m³)	Solution Application (Vol.) from Process Plant		
4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4,620	4.620	4.620	4.620	4 620	4,620	$(m^3)$	Initial Ore Water Inflow		
2,680	2,425	2,382	2,112	2,067	2,390	2,569	2,571	2,574	2,459	2,171	2,071	768	768	448	424	731	854	1,640	2,426	2,896	3,743	3,750	3,793	3,793	2,680	2,680	2,446	2,382	2,134	2,067	2,354	2,141	1,714	1.287	829	362	0	(m³)	Precip. on Irrigation & Draindown Cell Areas	Inflows	
447	404	397	352	344	398	428	428	429	410	362	345	128	128	75	71	122	142	273	404	483	624	625	632	632	447	447	408	397	356	344	392	428	428	429	415	362	362	(m³)	Precip.on Stacking Cell Area		
3,126					2,788	2,997	2,999	3,003	2,869	2,533	2,416	896	896	523	494	853	996	1,913	2,830	3,379	4,366	4,375	4,425	4,425		3,126	2,854								1.244	724	362	(m <sup>3</sup> )	Total Precip		Mass
421,236	420,939	420,889	420,574	420,521	420,898	421,107	421,109	421,113	420,979	420,643	420,526	419,006	419,006	418,633	418,604	418,963	419,106	420,023	420,940	421,489	422,476	422,485	422,535	422,535	421,236	421,236	420,964	420,889	420,600	420,521	420,856	420,679	339,402	258.126	176.804	95 434	14,222	(m <sup>3</sup> )	Total I		Mass Balance
10	3 -	C	0	0	0	0	12	29	341	1,123	1,212	2,365	2,365	3,220	3,286	3,460	3,529	3,213	2,897	2,587	2,029	1,879	977	977	10	10	2	0	0	0	0	0	7	14	88	187	0	(m <sup>3</sup> )	Evap. from Irrigation & Draindown Cell Area		_

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39	32	37	36	35	34	33	32	3	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	<u> </u>	10	9	œ	7	<b>о</b>	თ.	4	ωι	v	_		14-Day Cy	cle		٦
17-Mar	3-Mar	17-Feb	3-Feb	20-Jan	6-Jan	23-Dec	9-Dec	25-Nov	11-Nov	28-Oct	14-Oct	30-Sep	16-Sep	2-Sep	19-Aug	5-Aug	22-Jul	8-Jul	24-Jun	10-Jun	27-May	13-May	29-Apr	15-Apr	1-Apr	18-Mar	4-Mar	18-Feb	4-Feb	21-Jan	7-Jan	24-Dec	10-Dec	26-Nov	12-Nov	29-Oct	15-Oct	1-0ct		Start Dat	е		
			) 16-Feb																ı 7-Jul	1 23-Jun		ı			· 14-Apr	· 31-Mar												t 14-0ct		End Date	)		
18.56	18 56	16.79	16.50	14.63	14.32	16.55	17.79	17.81	17.83	17.03	15.04	14.34	5.32	5.32	3.11	2.94	5.06	5.92	11.36	16.80	20.06	25.92	25.97	26.27	26.27	18.56	18.56	16.94	16.50	14.78	14.32	16.30	17.79	17.81	17 83	17.23	15 04	15.04	(mm)	Precip. per Cycle	Cilillatic Data	Climati	
0.3	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.8	9.5	31.1	33.6	65.5	65.5	89.2	91.0	95.9	97.8	89.0	80.3	71.7	56.2	52.1	27.1	27.1	0.3	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.3	0.8	7.3	31 1	31.1	(mm)	Evap. per Cycle	Data	, Data	
154,000	154 000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154 000	154,000	154 000	154,000	(tonnes)	Ore Prod.	Operation Data	Operati	
6,006,000	5 852 000	5,698,000	5,544,000	5,390,000	5,236,000	5,082,000	4,928,000	4,774,000	4,620,000	4,466,000	4,312,000	4,158,000	4,004,000	3,850,000	3,696,000	3,542,000	3,388,000	3,234,000	3,080,000	2,926,000	2,772,000	2,618,000	2,464,000	2,310,000	2,156,000	2,002,000	1,848,000	1,694,000	1,540,000	1,386,000	1,232,000	1,078,000	924,000	770,000	616 000	462,000	308 000	154,000	(tonnes)	Cum. Ore Prod.	on Data	on Data	
21	2	0	0	0	0	0	0	2	5	57	187	202	394	394	537	548	577	588	536	483	431	338	313	163	163	2	2	0	0	0	0	0	0	2	ת	44	187	187	(m <sup>3</sup> )	Evap. from Stacking Cell Area			
11 :	11	2	0	0	0	0	0	14	33	398	1,310	1,414	2,759	2,759	3,757	3,834	4,036	4,117	3,749	3,380	3,018	2,368	2,192	1,140	1,140	11	1	2	0	0	0	0	0	9 ;	19	132	374	187	(m <sup>3</sup> )	Total ( Evap	Outriows	2	
18,480	18 480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	0	0	О	0	O	0	(m³)	Ore Water Outflow	OWS	Outs	
18,491	18 491	18,482	18,480	18,480	18,480	18,480	18,480	18,494	18,513	18,878	19,790	19,894	21,239	21,239	22,237	22,314	22,516	22,597	22,229	21,860	21,498	20,848	20,672	19,620	19,620	18,491	18,491	18,482	18,480	18,480	18,480	18,480	0	9 ;	19	132	374	187	(m <sup>3</sup> )	Total Outflow			
402,745	402 745	402,457	402,409	402,094	402,041	402,418	402,627	402,615	402,599	402,100	400,853	400,632	397,767	397,767	396,396	396,291	396,447	396,509	397,795	399,080	399,990	401,629	401,813	402,916	402,916	402,745	402,745	402,481	402,409	402,120	402,041	402,376	420,679	339,394	258 107	176,672	95 059	14,035	(m <sup>3</sup> )	Leached Solution to Pregnant Pond			
36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	36	0	0	0	0	0	0	0	(m³/hr)	Freshwater Flow Rate Required			
12,168	12 168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	12,168	0	0	0	O	0	O	0	$(m^3)$	Freshwater (Vol.) Required			
18,480	18 480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	0	0	0	О	0	О	0	(m <sup>3</sup> )	Ore Water Inflow			
893	893	808	794	704	689	797	856	857	858	820	724	690	256	256	149	141	244	285	547	809	965	1,248	1,250	1,264	1,264	893	893	815	794	711	344	0	0	0	0	0	0	0	(m <sup>3</sup> )	Precip. on Rinse & Unloading Cell Areas	Intiows	belows	
447	447	404	397	352	344	398	428	428	429	410	362	345	128	128	75	71	122	142	273	404	483	624	625	632	632	447	447	408	397	356	689	1,177	1,713	2,142	2 574	2,902	2 895	3,257	(m³)	Precip. on Contingency Area		Mass Dalalice	Mass B
1,340	1 340	1,212	1,191	1,056	1,033	1,195	1,284	1,285	1,287	1,229	1,086	1,035	384	384	224	212	366	427	820	1,213	1,448	1,871	1,875	1,897	1,897	1,340	1,340	1,223	1,191	1,067	1,033	1,177	1,713	2,142	2 574	2,902	2 895	3,257	(m³)	Total Precip		alalice 2	alanca 3
31,988	31 988	31,860	31,839	31,704	31,681	31,843	31,932	31,933	31,935	31,877	31,734	31,683	31,032	31,032	30,872	30,860	31,014	31,075	31,468	31,861	32,096	32,519	32,523	32,545	32,545	31,988	31,988	31,871	31,839	31,715	31,681	1,177	1,713	2,142	2 574	2,902	2 895	3,257	(m³)	Total Inflow			
ω	ω	0	0	0	0	0	0	4	10	114	374	404	788	788	1,073	1,095	1,153	1,176	1,071	966	862	676	626	326	326	3	ω	_	0	0	0	0	0	0	О	0	О	0	(m <sup>3</sup> )	Evap. on Rinse & Unloading Cell Areas			
18,480	18 480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	18,480	0	0	0	0	Э	0	O	0	(m <sup>3</sup> )	Ore Water Outflow	Swoimno	Outflow	
18,483	18 483	18,480	18,480	18,480	18,480	18,480	18,480	18,484	18,490	18,594	18,854	18,884	19,268	19,268					19,551	19,446	19,342	19,156	19,106	18,806	18,806	18,483	18,483	18,481	18,480	18,480	0	0	0	0	Э	0	Э	0	(m³)	Total Outflow	NS	5	
13,505	13 505	13,380	13,359	13,224	13,201	13,363	13,452	13,449	13,445	13,284	12,879	12,799	11,764	11,764	11,319	11,284	11,380	11,419	11,917	12,415	12,753	13,363	13,416	13,739	13,739	13,505	13,505	13,390	13,359	13,235	31,681	1,177	1,713	2,142	2 574	2,902	2 895	3,257	(m <sup>3</sup> )	Rinse Solution to Rinse Pond			

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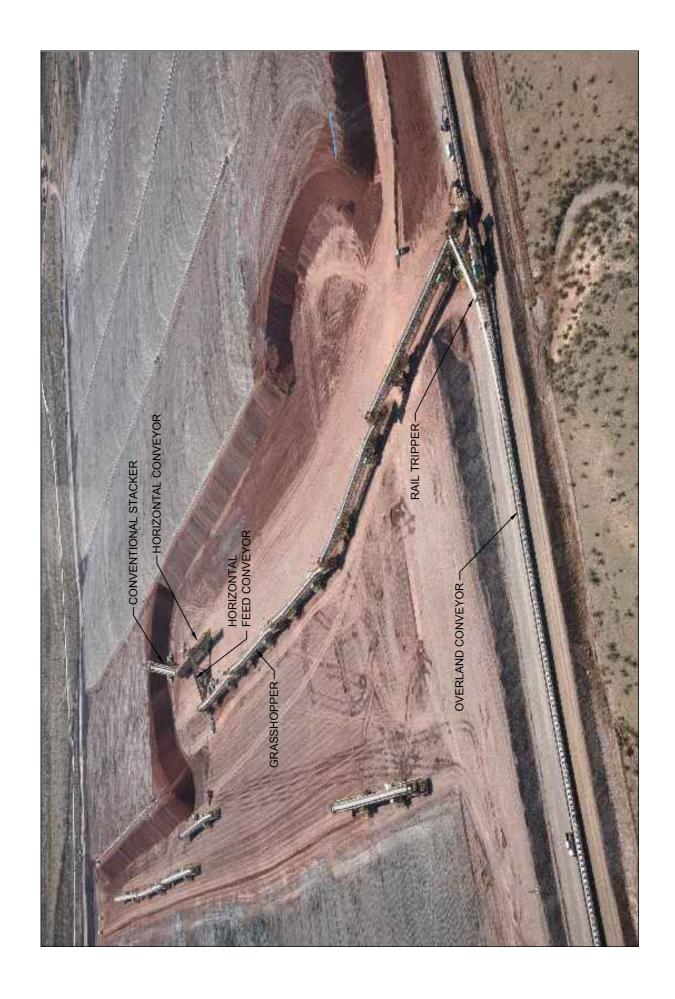
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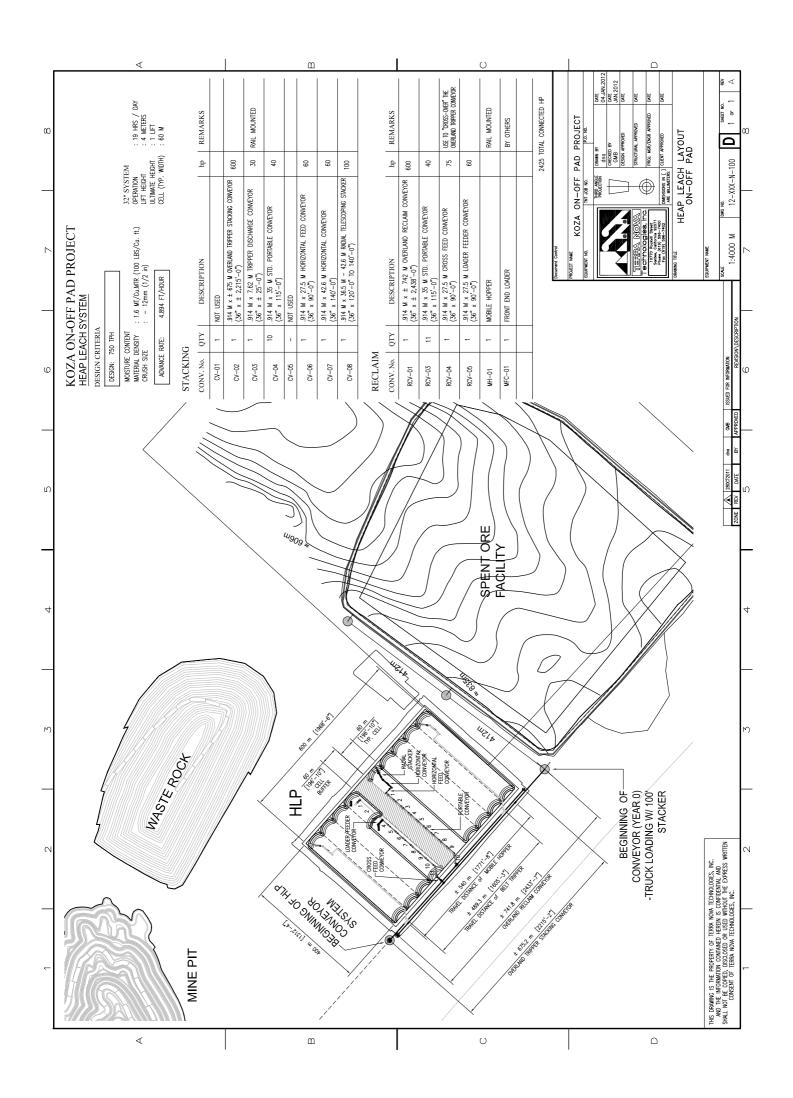
39	38	37	36	35	34	33	32	31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	1	10	9	œ	7	ח כ	4 n	ω	2	_		14-Day Cy	cle		
17-Mar	3-Mar	17-Feb	3-Feb	20-Jan	6-Jan	23-Dec	9-Dec	25-Nov	11-Nov	28-Oct	14-Oct	30-Sep	16-Sep	2-Sep	19-Aug	5-Aug	22-Jul	8-Jul	24-Jun	10-Jun	27-May	13-May	29-Apr	15-Apr	1-Apr	18-Mar	4-Mar	18-Feb	4-Feb	21-Jan	7-Jan	24-Dec	10-Dec	12-Nov	29-Oct	15-Oct	1-0ct		Start Dat	e		
		2-Mar	16-Feb		19-Jan		22-Dec	8-Dec		10-Nov			29-Sep				4-Aug		7-Jul	١.,		26-May	12-May		14-Apr								23-Dec						End Dat	е		
18.56	18.56	16.79	16.50	14.63	14.32	16.55	17.79	17.81	17.83	17.03	15.04	14.34	5.32	5.32	3.11	2.94	5.06	5.92	11.36	16.80	20.06	25.92	25.97	26.27	26.27	18.56	18.56	16.94	16.50	14.78	14.32	16.30	17.01	17.83	17.23	15.04	15.04	(mm)	Precip. per Cycle	Cilinatic Data	Climati	
0.3	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.8	9.5	31.1	33.6	65.5	65.5	89.2	91.0	95.9	97.8	89.0	80.3	71.7	56.2	52.1	27.1	27.1	0.3	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.8	7.3	31.1	31.1	(mm)	Evap. per Cycle	Data	, Data	
154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	(tonnes)	Ore Prod.	operau	Opporati	
6,006,000	5,852,000	5,698,000	5,544,000	5,390,000	5,236,000	5,082,000	4,928,000	4,774,000	4,620,000	4,466,000	4,312,000	4,158,000	4,004,000	3,850,000	3,696,000	3,542,000	3,388,000	3,234,000	3,080,000	2,926,000	2,772,000	2,618,000	2,464,000	2,310,000	2,156,000	2,002,000	1,848,000	1,694,000	1,540,000	1,386,000	1,232,000	1,078,000	924 000	616,000	462,000	308,000	154,000	(tonnes)	Cum. Ore Prod.	Operation Data		
1,198.6	1,198.6	1,197.8	1,197.6	1,196.7	1,196.6	1,197.7	1,198.3	1,198.3	1,198.2	1,196.7	1,193.0	1,192.4	1,183.8	1,183.8	1,179.8	1,179.4	1,179.9	1,180.1	1,183.9	1,187.7	1,190.4	1,195.3	1,195.9	1,199.2	1,199.15	1,198.6	1,198.6	1,197.9	1,197.6	1,196.8	1,196.6	1,197.5	1 252 0	768.2	525.8	282.9	41.8	(m³/hr)	Daily Solution Flowrate from Pad			
0.14	0.14	0.12	0.12	0.11	0.11	0.12	0.13	0.13	0.13	0.13	0.11	0.11	0.04	0.04	0.02	0.02	0.04	0.04	0.08	0.13	0.15	0.19	0.19	0.20	0.20	0.14	0.14	0.13	0.12	0.11	0.11	0.12	013	0.13	0.13	0.11	0.11	(m³/hr)	Pond Precip.	Intiows	26000	
1,199	1,199	1,198	1,198	1,197	1,197	1,198	1,198	1,198	1,198	1,197	1,193	1,192	1,184	1,184	1,180	1,179	1,180	1,180	1,184	1,188	1,191	1,196	1,196	1,199	1,199	1,199	1,199	1,198	1,198	1,197	1,197	1,198	1,010	768	526	283	42	(m³/hr)	Total Inflow			
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.2	0.2	0.5	0.5	0.7	0.7	0.7	0.7	0.7	0.6	0.5	0.4	0.4	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.2	0.2	(m³/hr)	Pond Evap.	out	1891	Drogn
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.2	0.2	0.5	0.5	0.7	0.7	0.7	0.7	0.7	0.6	0.5	0.4	0.4	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.2	0.2	(m³/hr)	Total Outflow	Outriows	Outflows	Dono
7192.7	7192.7	7187.5	7186.6	7180.9	7179.9	7186.8	7190.6	7190.3	7190.0	7180.7	7157.4	7153.3	7100.3	7100.3	7074.7	7072.7	7075.4	7076.4	7100.0	7123.6	7140.4	7170.6	7174.1	7194.9	7194.9	7192.7	7192.7	7187.9	7186.6	7181.4	7179.9	7186.0	75129	4609.8	3155.3	1696.8	249.9	(m³)	Pond Vol. for 2-m head	Operation	֓֡֜֜֜֜֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֜֓֡֓֜֓֓֓֡֓֜֓֜֓֡֓֡֓֡֓֡֓֡֓֡֓֡֓֡֡֡֓֜֡֓֜	
7,193	7,193	7,187	7,187	7,181	7,180	7,187	7,191	7,190	7,190	7,181	7,157	7,153	7,100	7,100	7,075	7,073	7,075	7,076	7,100	7,124	7,140	7,171	7,174	7,195	7,195	7,193	7,193	7,188	7,187	7,181	7,180	7,186	7 513	4,610	3,155	1,697	250	(m³)	Daily Pond Storage			
1,199	1,199	1,198	1,198	1,197	1,197	1,198	1,198	1,198	1,198	1,197	1,193	1,192	1,183	1,183	1,179	1,179	1,179	1,179	1,183	1,187	1,190	1,195	1,196	1,199	1,199	1,199	1,199	1,198	1,198	1,197	1,197	1,198	1 252	768	526	283	42	(m³/hr)	to Process Plant	Not Inflow		
1,199	1,199	1,198	1,198	1,197	1,197	1,198	1,198	1,198	1,198	1,197	1,193	1,192	1,183	1,183	1,179	1,179	1,179	1,179	1,183	1,187	1,190	1,195	1,196	1,199	1,199	1,199	1,199	1,198	1,198	1,197	1,197	1,198	1,010	768	526	283	42	(m³/hr)	Pregnant Pond			
32	32	33	33	34	34	33	32	32	32	34	38	38	47	47	52	52	51	51	47	43	41	36	35	31	31	32	32	33	33	34	34	3	-20	-19	-17	-15	-14	(m³/hr)		Intiows	la flower	
1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1 230 6	749.4	508.8	268.1	27.5	(m³/hr)	Total Inflow		-	Droc
27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	27.5	(m³/hr)	To Aggl.		633 1 10	Process Plant
1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	1203	722	481	241	0	(m³/hr)	To Leach Cells	Swound	0	1
1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1,230.6	1 230 6	749.4	508.8	268.1	27.5	(m³/hr)	Total Outflow			
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(m³/hr)	Net Inflow			
40	40	40	40	39	39	40	40	40	40	40	38	38	35	35	34	34	34	34	35	37	38	40	40	41	41	40	40	40	40	39	94	4	лС	0 00	9	9	10	(m³/hr)				
0.9	0.9	0.8	0.8	0.7	0.7	0.8	0.9	0.9	0.9	0.8	0.7	0.7	0.3	0.3	0.2	0.1	0.3	0.3	0.6	0.8	1.0	1.3	1.3	1.3	1.3	0.9	0.9	0.8	0.8	0.7	0.7	0.8	0.9	0.9	0.9	0.7	0.7	(m³/hr)		Intiows	261000	
41	41	41	41	40	40	41	41	41	41	40	39	39	35	35	34	34	34	34	36	38	39	41	41	42	42	41	41	41	41	40	95	4	n ~	9	9	9	10	(m³/hr)	1			
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	1.5	1.7	3.3	3. 3	4.4	4.5	4.8	4.9	4.4	4.0	3.6	2.8	2.6	1.3	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.4	<u>1</u> .5	1.5	(m³/hr)	Pond Evap.			

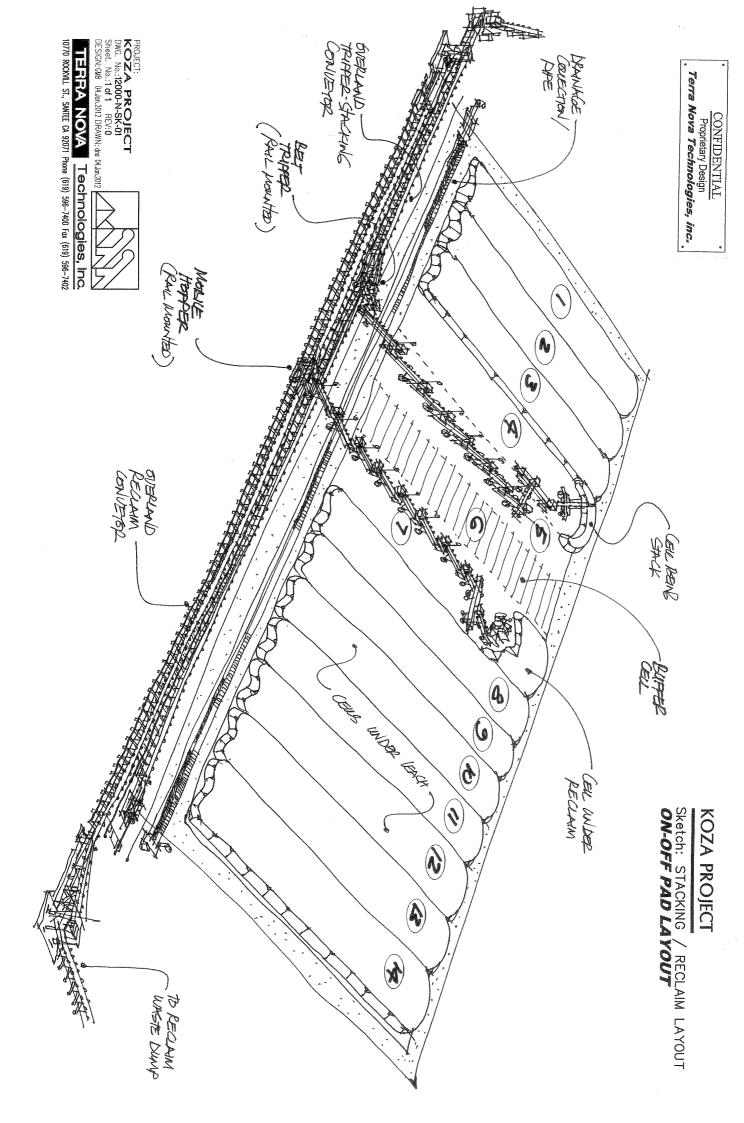
S	38	37	36	35	34	33	32	<u>ω</u>	30	29	28	27	26	25	24	23	22	21	20	19	8	17	16	15	4	3	12	=	10	9	ω ·	7 0	9 0	4	ω	2	_		14-Day Cyc	le	
I/-IVIdi	3-Mar	17-Feb	3-Feb	20-Jan	6-Jan	23-Dec	9-Dec	25-Nov	11-Nov	28-Oct	14-0ct	30-Sep	16-Sep	2-Sep	19-Aug	5-Aug	22-Jul	luL-8	24-Jun	10-Jun	27-May	13-May	29-Apr	15-Apr	1-Apr	18-Mar	4-Mar	18-Feb	4-Feb	21-Jan	7-Jan	24-Dec	26-Nov	12-Nov	29-Oct	15-Oct	1-0ct		Start Date	,	
JO-IVIAI		2-Mar	16-Feb	2-Feb	19-Jan	5-Jan	22-Dec									18-Aug	4-Aug	21-Jul	7-Jul			26-May	12-May	28-Apr	14-Apr	31-Mar	17-Mar	3-Mar	17-Feb	3-Feb		6-Jan	9-Dec	25-Nov	11-Nov	28-Oct	14-Oct		End Date		
10.00	18.56	16.79	16.50	14.63	14.32	16.55	17.79	17.81	17.83	17.03	15.04	14.34	5.32	5.32	3.11	2.94	5.06	5.92	11.36	16.80	20.06	25.92	25.97	26.27	26.27	18.56	18.56	16.94	16.50	14.78	14.32	16.30	17.81	17.83	17.23	15.04	15.04	(mm)	Precip. per Cycle	Climatic Data	
0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.8	9.5	31.1	33.6	65.5	65.5	89.2	91.0	95.9	97.8	89.0	80.3	71.7	56.2	52.1	27.1	27.1	0.3	0.3	0.1	0.0	0.0	0.0	0.0	0.3	0.8	7.3	31.1	31.1	(mm)	Evap. per Cycle	c Data	
104,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154,000	154.000	154,000	154,000	154,000	154,000	154,000	154,000	(tonnes)	Ore Prod.	Operat	-
0,000,000	5,852,000	5,698,000	5,544,000	5,390,000	5,236,000	5,082,000	4,928,000	4,774,000	4,620,000	4,466,000	4,312,000	4,158,000	4,004,000	3,850,000	3,696,000	3,542,000	3,388,000	3,234,000	3,080,000	2,926,000	2,772,000	2,618,000	2,464,000	2,310,000	2,156,000	2,002,000	1,848,000	1,694,000	1,540,000	1,386,000	1.232.000	1.078.000	770,000	616,000	462,000	308,000	154,000	(tonnes)	Cum. Ore Prod.	Operation Data	
20	32	33	33	34	34	33	32	32	32	34	38	38	47	47	52	52	51	51	47	43	41	36	35	31	31	32	32	33	33	34	34	33	(20)	(19)	(17)	(15)	(14)	$(m^3/hr)$	To Process Plant	Outflows	
20	32	33	33	34	34	33	32	32	32	34	39	40	47	47	52	52	51	51	47	43	41	38	38	33	33	32	32	33	33	34	34	33	-20	-19	-17	-13	-13	(m <sup>3</sup> /hr)	Total Outflow	6	Rinse Pond
ď	9	8	8	6	6	8	9	9	9	6	0	0	0	0	0	0	0	0	0	0	0	ω	4	9	9	9	9	8	8	6	<u></u>	0 6	27	27	26	22	23	(m³/hr)	Net Inflow		Pond
7.777	222.2	190.7	185.4	150.7	144.9	186.4	209.4	207.9	205.9	145.7	26.2	26.2	26.2	26.2	26.2	26.2	26.2	26.2	26.2	26.2	26.2	65.6	88.4	224.9	224.9	222.2	222.2	193.3	185.4	153.5	1465	26.2	659.5	658.4	630.4	539.7	553.0	(m <sup>3</sup> )	Operating Pond Vol. for 2-m head	:	
777	222	191	185	151	145	186	209	208	206	146	0	0	0	0	0	0	0	0	0	0	0	66	88	225	225	222	222	193	185	154	1465	0	659	658	630	540	553.0	$(m^3)$	Daily Pond Storage		
c	0	0	0	0	0	0	0	0	0	0	0	0	12	12	18	18	17	17	11	6	2	0	0	0	0	0	0	0	0	0	0	29	0	0	0	0	0	(m³/hr)	Make-Up Volume		
y	9	8	œ	6	6	8	9	9	9	6	0	0	0	0	0	0	0	0	0	0	0	ω	4	9	9	9	9	œ	œ	6	67	0	0	0	0	0	0	(m <sup>3</sup> /hr)	Excess Pond Volume		

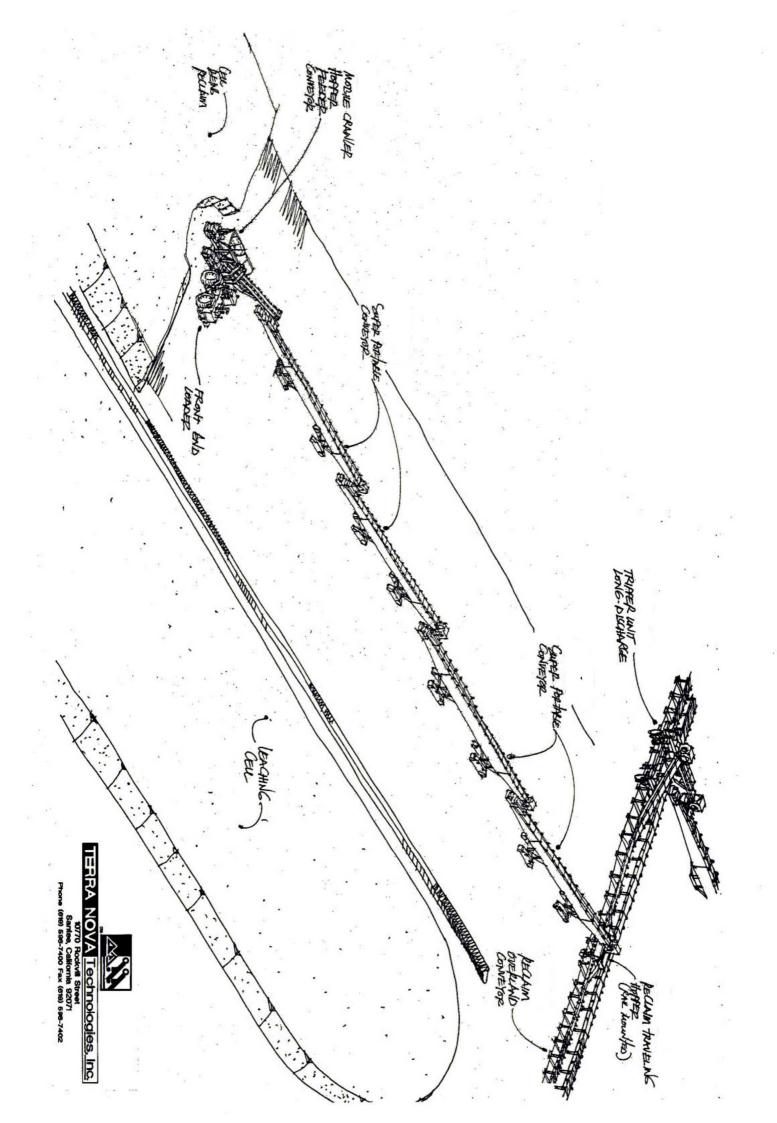
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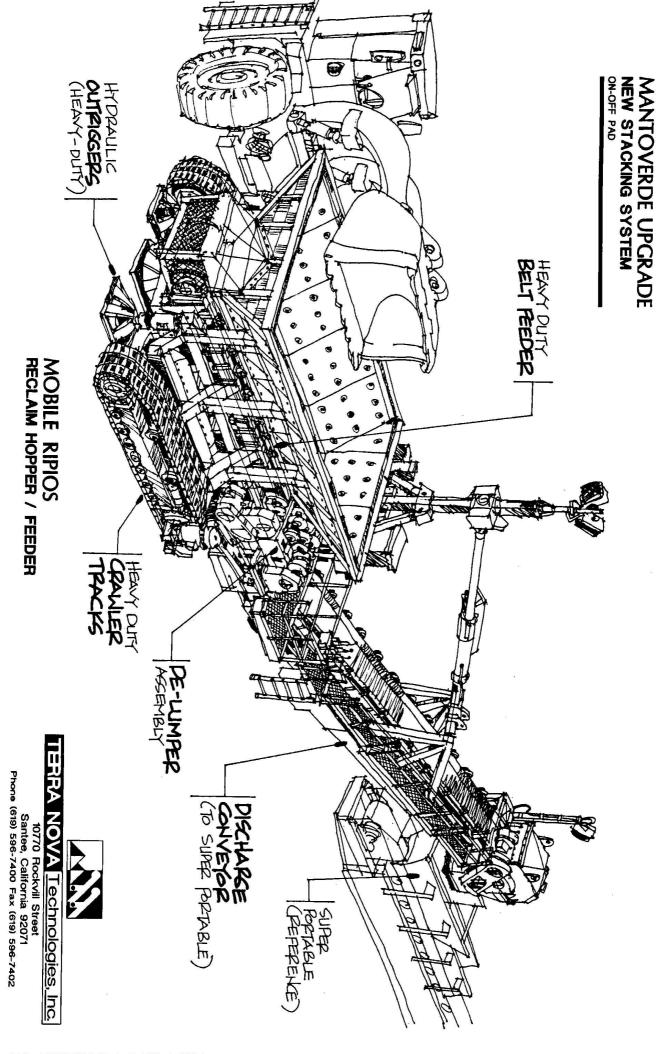
Appendix D: Stacking and Reclaiming Vendor Information

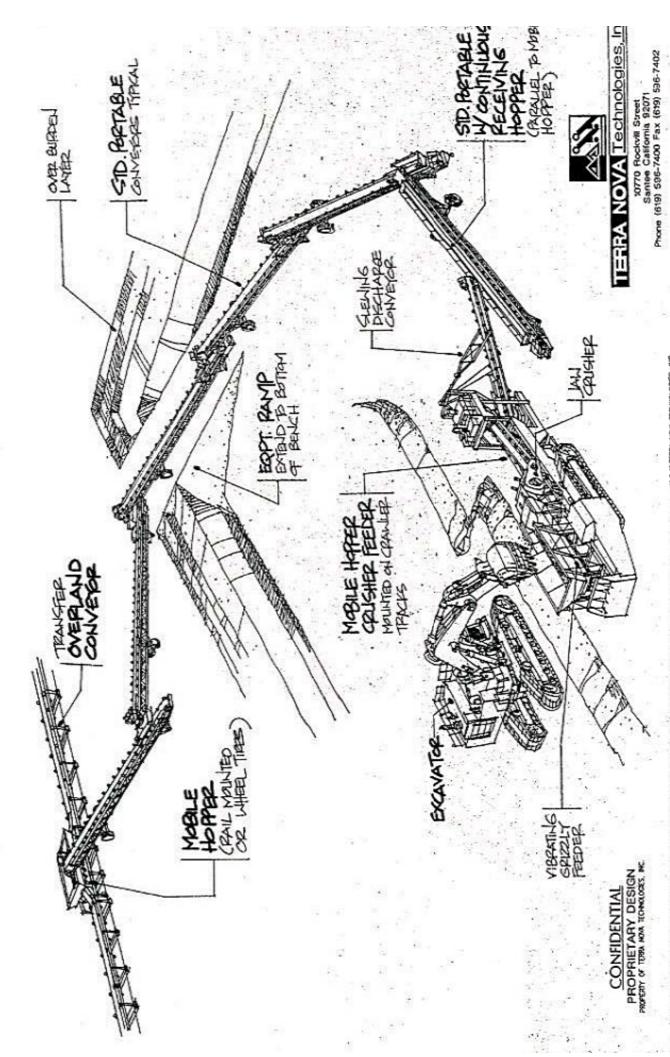




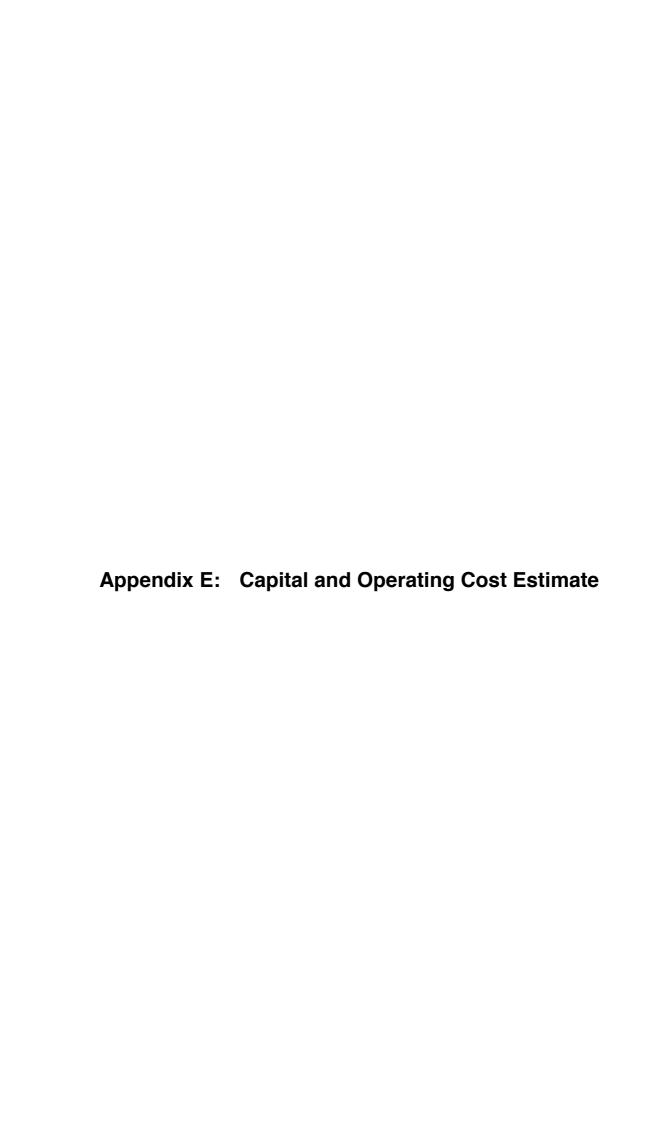








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**Appendix E-1:Unit Cost Summary** 



Table E-1: Unit Cost Summary

Item No.	Item	Unit Cost	Units	Contingency
100	•			
	Mobilization and Demobilization	10%	ls	40%
	Clear and Grub	\$3,500	ha	40%
	Topsoil Removal and Stockpiling	\$1.50	m <sup>3</sup>	40%
	Earthworks	***	3	
	HLP Overexcavation	\$3.00	m³	40%
	SOF Overexcavation	\$3.00	m <sup>3</sup>	40%
	HLP Compacted Fill: Regrading	\$1.50	m <sup>3</sup>	40%
	SOF Compacted Fill: Regrading	\$1.50	m <sup>3</sup>	40%
	HLP Subgrade Preparation	\$0.25	m <sup>2</sup>	40%
	SOF Subgrade Preparation	\$0.25	m²	40%
	HLP Soil Liner	\$5.00	m³	40%
	SOF Soil Liner	\$6.00	m <sup>3</sup>	40%
	HLP Single Anchor Trench	\$2.50	lm	40%
	SOF Single Anchor Trench	\$2.50	lm	40%
	HLP Double Anchor Trench	\$6.00	lm 3	40%
	HLP Blasting	\$3.00	m <sup>3</sup>	40%
	SOF Blasting	\$3.00	m <sup>3</sup>	40%
270				40%
271				40%
	Geosynthetics		2	
	HLP 2.0mm HDPE Geomembrane	\$6.20	m <sup>2</sup>	35%
	HLP 1.5mm HDPE Geomembrane	\$4.95	m <sup>2</sup>	35%
321	SOF 1.5mm HDPE Geomembrane	\$4.95	m²	35%
325	HLP 1.5mm HDPE Geomembrane (Double) and Drain Net	\$6.19	m <sup>2</sup>	35%
400	Overliner			
410	HLP Overliner	\$15.00	m <sup>3</sup>	40%
411	SOF Overliner	\$16.00	m <sup>3</sup>	40%
500	Piping			
510	Drainage Piping	\$1.15	m²	35%
600	Diversion Channel			
610		\$0.00	m²	40%
	Miscellaneous Mechanical Equipment			
	Fixed Conveyor	\$2,900	lm	30%
	Rail tripper	\$655,000	LS	30%
	Mobile Stacking Conveyor w/ tripper	\$5,700,000	LS	30%
	Bucket wheel	\$3,000,000	LS	30%
	Mobile Reclaim Conveyor w/ hopper	\$5,200,000	LS	30%
	Rail Hopper	\$343,000	LS	30%
	Mobile Conveyor Hopper	\$1,300,000	LS	30%
	Grasshopper	\$148,000	LS	30%
	Horizontal Feed Conveyor	\$244,000	LS	30%
	Horizontal Conveyor Conventional Stacker	\$694,000 \$774,000	LS	30%
		\$774,000	LS	30%
	Radial Stacker	\$465,000	LS	30%
	20 t Truck	\$375,000 \$730,000	EΑ	30%
	958 Loader 12M Grader	\$730,000	EA EA	30%
		\$320,000		30%
	20m³ Water truck D7 Dozer	\$275,000 \$570,000	EA EA	30% 30%
	Overland Conveyor	\$2,350	LM	30%
	Closure	Ψ2,330	LIVI	30 /0
	Mobilization and Demobilization	10%	ls	40%
	Regrading	\$0.50	m <sup>2</sup>	40%
	Rock Armour Placement	\$13.50	m <sup>3</sup>	40%
	Topsoil Placement		m <sup>3</sup>	40%
	Structure Removal Conveyors	\$1.80 \$40	m	40%
	General Labor Crew	\$1,200	•••	40%
	Construction and Engineering	Ų.,200		1070
	Engineering	2%	ls	25%
920	CM	5%	ls	25%
	Owner Costs	5%	ls	25%
	OPEX			
	Stacker / Conveyor / Reclaimer OPEX	\$0.55	per tonne	
	Overland Conveyor	\$0.09	per t-km	
	Grasshoppers and Stacker	\$0.24	per tonne	
	Loader Truck and Loader	\$0.12 \$0.85	per tonne	
	Truck and Loader Truck and Grasshopper OPEX	\$0.85 \$1.02	per tonne	
	Electrical Power	\$0.07	per tonne per kw-hr	
1040		\$2.10	per litre	
	Grasshopper / Stacker OPEX	\$0.53	per tonne	



## 

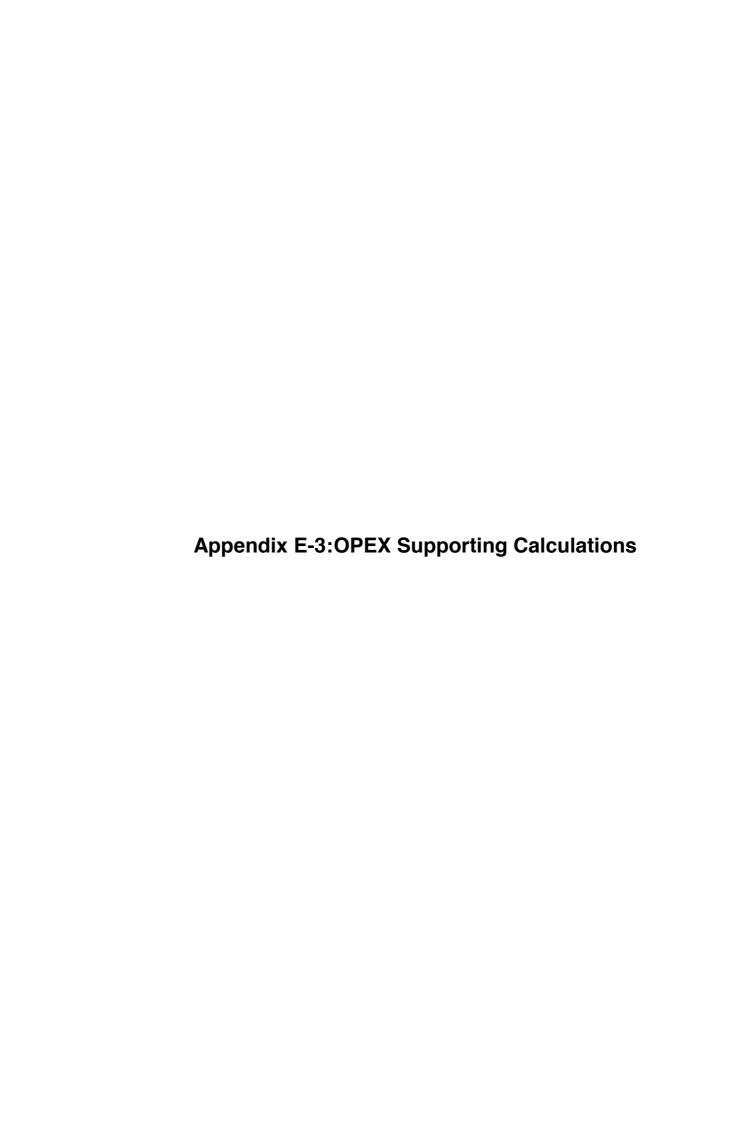
# Himmetdede Gold HLP Project Table E-2: On Off HLP PFS Cost Estimate With Grasshoppers, Loader and Conveyors Quantity Summary

Year Year 1 2 Quantity Quantity	Cuantity Summary   Year   Ye	Cuantity Summary   Vear   Ve
Year Year  1 2  Quantity Quantity  1 136  1383346 111,770 1004,28 113,523 100,628 113,526 113,526 113,526 113,527 113,528 113,	Year Year Year 1 1 2 3 3 4 1 2 1 3 1 1 2 1 3 1 1 1 2 1 3 1 1 1 1 1	Year Year Year Year Year 1 2 3 4 4 4 1 2 1 3 4 4 4 1 1 2 1 3 4 4 1 1 2 1 3 4 4 1 1 2 1 3 1 4 1 1 2 1 3 1 4 1 1 2 1 3 1 4 1 1 2 1 3 1 4 1 1 2 1 3 1 4 1 1 2 1 3 1 3
	Year 3 Quantity	Year Year 3 4 Quantity Quantity
		Year 4 Quantity



## Himmetdede Gold HLP Project Table E-2: On Off HLP PFS Cost Estimate With Grasshoppers, Loader and Conveyors Cost Summary

1014	1011	1012	1013	7.01	1011					930	920	910	900	841	840	830	820	800	790	784	782	781	780	771	767	766	765	760	750	740	720	710	610	600	510	411	410					270	261	255	251	241	240	231	230	220	211	200	130	110	iem No.	No most
Truck and Loader	Overland Conveyor	Grasshoppers and Stacker	Loader	OPEX (Spent Ore Removal)	Overland Conveyor	OPEX (Ore Stacking)	OPEX Total	Annual Tonnage	Sub Total Contingency	Owner Costs	CM	Engineering	Construction and Engineering	Structure Removal Conveyors	Topsoil Placement	Rock Armour Placement	Regrading	Mobilization and Demobilization	Overland Conveyor	D7 Dozer	12M Grader	958 Loader	20 t Truck	Padial Stacker	Horizontal Conveyor	Horizontal Feed Conveyor	Grasshopper	Rail Hopper	Mobile Reclaim Conveyor w/ hopper	Bucket wheel	Rail tripper	Fixed Conveyor	N/A	Diversion Channel	Piping Drainage Piping	SOF Overliner	Overliner HLP Overliner	HLP 1.5mm HDPE Geomembrane (Double) and Drain Net	HLP 1.5mm HDPE Geomembrane SOF 1.5mm HDPE Geomembrane	HLP 2.0mm HDPE Geomembrane	Geosynthetics	N/A	SOF Blasting	HLP Double Anchor Trench	SOF Single Anchor Trench	HI D Single Anchor Trench	HLP Soil Liner	SOF Subgrade Preparation	HLP Subgrade Preparation	HLP Compacted Fill: Regrading	SOF Overexcavation	HI P Overexcevation	Topsoil Removal and Stockpiling	Mobilization and Demobilization	City Decemention	ltom
				pertonne	per t-km			t a	Ŗ	ls	ıs	ls.	uay	a a	я,	m <sub>3</sub>	3, ∂	0	lm	e e	ea	ea	ea -	· 6	8	Is	<u>s</u> s	- <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> - <del>-</del> <del>-</del> -	m²		3,	в,	3.	m²	a, a	, ∄,			3 3		3 :	3 3	L 3	3.	<b>a</b> ₂ :	3, 3,	3, 3	₹.	na ha	ভ	Onks	linite				
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\$ 24,779,774	\$ 853,160	\$ 6,891,112	3.535.855	\$ 36.059.902	3,816,354	\$ 10,707,467	\$ 46,767,368	\$ 29.200,000	n <b>c</b> n	\$ 2,865,591	\$ 2,865,591	\$ 1,146,236	\$ 6.877.418	\$ 93,400 \$	\$ 546,455	\$ 1,782,486	\$ 638.013	o •o	\$ 2,220,750	\$ 1,140,000 \$	\$ 320,000	69	\$ 750,000 \$	n e	69	69	\$ 3,108,000	\$ 343,000	-	· · ·	\$ 655,000	4.4			9 314,599 \$	\$ 13,048,085	en en	\$ 118,082	\$ 4,485,279	•		n en	300,100	\$ 4,512	\$ 9,058 \$	1,631,010		\$ 226,529	\$ 75,663	\$ 353,094	\$ 32,034 \$	\$ 3,516,015 \$	\$ 612,642		n	Cost
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\$ 1,018,347	\$ 48,29	\$ 283,196	\$ 145.30		\$ 156,836		\$ 1,935,177	1,200,00	\$ 3,770,35	\$ 168,31	\$ 168,31	\$ 67,32	\$ 403.96	\$ 93,400	\$ 546,45	\$ 1,782,48	\$ 638.01	\$ 3,366,38	-			·	69 6			69			69													n en				n 41		69 1		n 6n	• •				s cost	Cost





Operating	equipment	cost esti	mate is	as follows	:
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Operating equipment cost estimate is as follows									
Fixed CV	\$	0.044	per tonne					750	
Rail tripper	\$	0.014	per tonne						
MSC w/ tripper	\$	0.093	per tonne						
Bucket wheel	\$	0.143	per tonne						
MRC w/ hopper	\$	0.071	per tonne						
Rail hopper	\$	0.014	per tonne						
1000m Fixed CV	\$		per tonne					640	
Subtotal System	\$		per tonne						
	*		p = 101					1390	
OPEX (Grasshopper)									
			n						
Maintenance (parts, mechanical, tires, lube, etc.)	\$	2.240		10.5	\$	23.52			per hr
Labor (repair)	\$	0.090		10.5		0.95			per hr
Repair Crew (4 persons)	\$	15.000			\$	15.00			per hr
Subtotal System	\$	17.330			\$	39.47			per hr
oubtotal Gystem	Ψ	17.000			Ψ	00.47			portin
		kw	n			kwh			
Electrical Load		20		23		460	\$	32 20	per hour
Electrical Edda		20		20		400	Ψ	02.20	pernour
Sub total per hour	\$	71.67							
Operating hours per day	Ψ	18							hrs
Operating flours per day	\$	1,289.97							per day
	φ	1,209.91							per day
Grasshopper Cost (8 units)	\$	0.129							per tonne
Grasshopper Cost (29 units)	\$	0.468							per tonne
Grasshopper Cost (29 units)	Ψ	0.400							per torine
Overland conveyor OPEX	(	Overhaul	Maintena	ance		Lube	For 4	100 ft he	elt at 600 tph
eventure conveyor or EX	•	3.22		2.33		1.33	1 01 -	6.88	
		5.22		2.00		1.00	\$		per hour km at 600 tph
		0 0040	\$/t-km				Ψ	JU.72	
Overland belt lengths		0.0340	Ψ/ (-ΚΙΤΙ						
App1- On off									
		0							
App2 Grasshopper		U							
Crassbanner and Stocker Coat	¢	0.24							
Grasshopper and Stacker Cost	\$	0.24							
App2 Grasshopper									
Cub total Appd	•	0.550							
Sub total App1	\$	0.552							
Overland	\$	-	A II O						
TOTAL App1	\$	0.552	All Convey	ors/					
	•	0.40-							
Grasshoppers	\$		Grasshop	oers or	nıy				
Feed belt	\$	0.044							
TOTAL App2	\$	0.173	Add to true	ck haul	lage	e costs			
Grasshoppers (29 units)	\$		Grasshop	pers or	nly				
Feed belt	\$	0.058							
TOTAL App3	\$	0.526	Grasshop	oer OP	PEX	•			

**Appendix F: Liner Leakage Rate** 



## Flow Rate through Composite Liner h=0.3m

Equation: 
$$Q = 0.21 * a^{0.1} * h^{0.9} * k_s^{0.74}$$

(Assume "good' contact conditions)

**Input:** 
$$a = 1 cm^2 \rightarrow a = 1.00E-04 m^2$$

$$ks = 1.00E-06$$
 cm/s  $\rightarrow ks = 1.00E-08$  m/s

Output: 
$$Q = 3.40E-08 \text{ m}^3/\text{acre/s}$$

Calculation: Area = 
$$24,063$$
 m<sup>2</sup> m<sup>2</sup>

**Descriptions:** Q leakage rate through a hole in the geomembrane component (m^3/s)

a area of a circular hole in geomembrane (m^2)

h liquid head on top of the geomembrane (m)

k<sub>s</sub> hydraulic conductivity of the soil in the composite liner (m/s)

n number of holes per acre

Conversions: 1 m<sup>3</sup> 264.2 gal

1 day 86400 sec 1 acre 4047 m^2